

NOTICE OF MEETING TRAFFIC AND TRANSPORTATION COMMISSION

Jude Abanulo, Chair

Garrett Clemons
Thomas Gibney
Cynthia Griffiths
Gerald Holtz
Alan Kaplan
Jeremy Martin
Mike Stein
Melvin Willis

Rockville City Hall Black Eyed Susan Conference Room Tuesday, March 22, 2016 at 7:30 PM

AGENDA

7:30 - 7:35	1.	General Announcements, Introductions, and Public Comments
7:35 – 7:40	2.	Research Row Development(1401, 1405 & 1413 Research Blvd) : Staff Introduction
7:40 – 8:00	3.	Research Row Development(1401, 1405 & 1413 Research Blvd) : Applicant Presentation
8:00 – 8:20	4.	Research Row Development(1401, 1405 & 1413 Research Blvd) : Commission Discussion
8:20 – 8:45	5.	Role of the Commission
8:45 – 8:50	6.	Review and Approve February 2016 Meeting Minutes
8:50 – 8:55	7.	Staff Report & Updates
8:55 – 9:00	8.	Additional Items/ Discussion

Next Meeting: Tuesday, April 26, 2016 at 7:30 PM

RESEARCH ROW

1401, 1405, & 1413 RESEARCH BOULEVARD (NATIONAL CAPITAL RESEARCH PARK)

COMPREHENSIVE TRANSPORTATION REVIEW

COMPONENT 1 INTRODUCTION

1.1 Site Location and Project Overview

This report has been revised based on comments and feedback received from City Staff regarding the previously revised study submission, dated March 11, 2016. Point-by-point responses to City comments on the initial study submission are provided in Appendix A.

This report provides a Comprehensive Transportation Review (CTR) for the proposed redevelopment of the National Capital Research Park site located within the City of Rockville, Maryland. The Applicant, FP Research Boulevard, LLC on behalf of ARE/FP-Research Boulevard, LLC and ARE Research Group No. 8 Corp, is currently seeking approval of a level 2 site plan application that would allow for the partial redevelopment of the site with the addition of new retail and office uses (Research Row).

The existing three-story building located at 1405 Research Boulevard consists of approximately 73,000 SF of office space and would remain with the proposed development plan. The former 105,000 SF three-story Research and Development (R&D) building located at 1413 Research Boulevard was recently demolished in April of 2015 in preparation of the planned redevelopment. The original approval for the site also included an additional office building within the 1401 Research Boulevard lot for which APFO approval has since expired.

1.2 Site Overview

The site is conveniently situated along the north side of West Montgomery Avenue (MD 28) and west of Interstate 270 where a full-movement interchange provides regional access to both the north and south through Rockville and Montgomery County. An aerial graphic showing the site location and the adjacent road network is provided as Figure 1-1.

The property is currently zoned Mixed-Use-Employment (MXE) per the City of Rockville's Zoning Ordinance, and the proposed redevelopment includes a mix of office and retail uses consistent with those uses allowed within the MXE zone. Therefore, no changes to the existing zoning for the property are proposed with this application. Figure 1-2 illustrates the zoning for the subject site and the area surrounding the property.

Figure 1-1

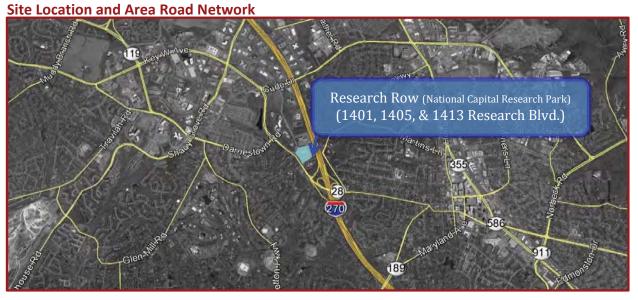
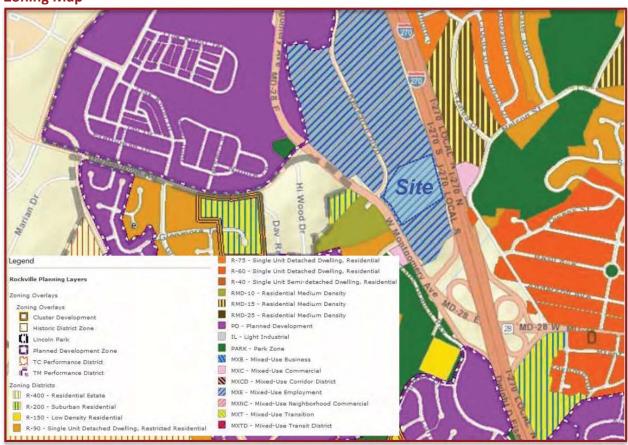


Figure 1-2 **Zoning Map**



Transportation Consultants INNOVATION + SOLUTIONS

1.3 Existing Land Use

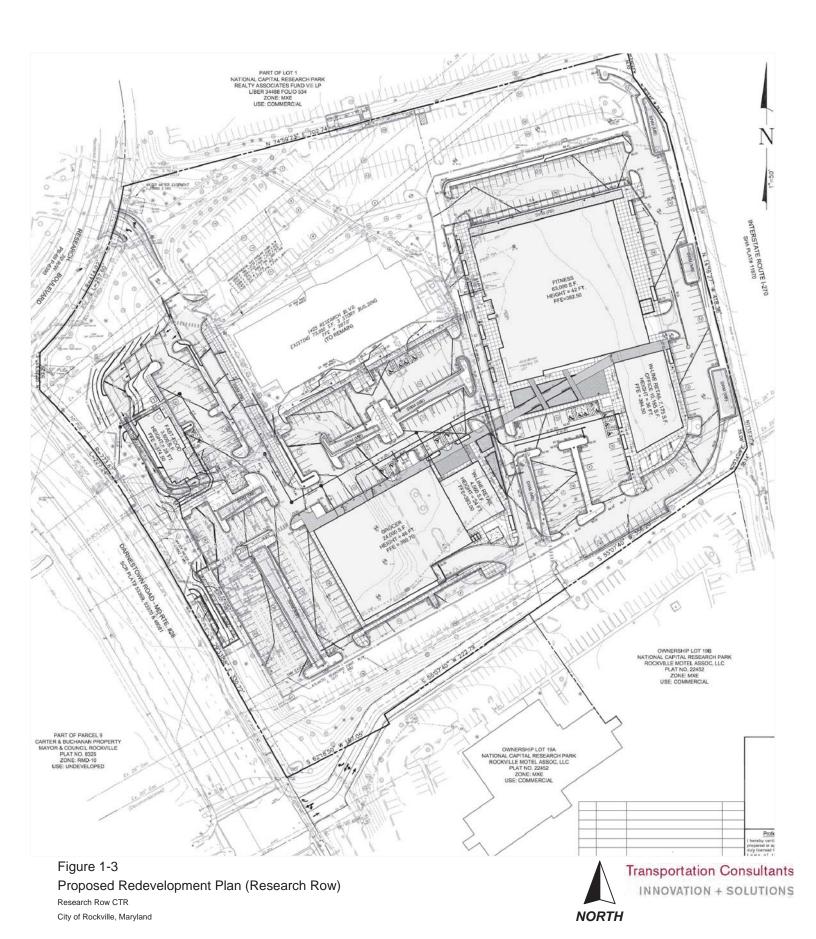
The overall site (1401, 1405, & 1413) consists of approximately 13.38 acres of land area with one existing three-story building located at 1405 Research Boulevard providing approximately 73,000 SF of office uses. The 1413 Research Boulevard lot was recently occupied by a 105,000 SF R&D building that was demolished in April of 2015, and the 1401 Research Boulevard lot is currently improved with a surface parking lot.

1.4 Proposed Land Use

The 1401 and 1413 Research Boulevard lots are proposed to be fully redeveloped with a mix of new retail and office uses.

A reduced copy of the proposed development plan is provided on Figure 1-3.

Note: The site trip generation calculated during the scoping process was based on a preliminary mix of 117,600 SF of retail uses and 15,300 SF of new office uses; however, the current plan includes a reduced development mix of up to 102,535 SF of GLA for retail uses and 10,165 SF of new office uses. Since the proposed plan would generate fewer trips, the study area would not increase beyond that which was identified during the scoping process. The existing 73,000 SF office building is to remain, and the 105,000 SF R&D building has been demolished.



See Table 1-1

Figure 1-4
Thresholds for Mitigation Requirements (CTR Guidelines - Table 3)

	Table 3: Mitigation Requirements
New Peak Hour Site Trips	Requirement
0 – 29	None
30 – 124	Transportation Improvement Fee Intersection Mitigation if exceeds Intersection Impact Thresholds
125 – 349	Transportation Improvement Fee Intersection Mitigation if exceeds Intersection Impact Thresholds Trip Reduction Plan consistent with Trip Reduction Tool for Office Uses
350 +	Transportation Improvement Fee Intersection Mitigation if exceeds Intersection Impact Thresholds Trip Reduction Plan consistent with Trip Reduction Tool for Office Uses Transportation Improvement Contribution consistent with Multimodal Analysis

The results of the baseline trip generation estimates are summarized on Table 1-1 and indicate that the proposed new retail and office uses would generate approximately 206 total trips during the weekday AM peak hour, 784 total trips during the weekday PM peak hour, and 894 total trips during the mid-day peak hour on Saturday.

The 105,000 SF R&D building (recently demolished) would generate approximately 136 total trips during the weekday AM peak hour, 138 total trips during the weekday PM peak hour, and 29 total trips during the mid-day peak hour on Saturday.

Table 1-1 Baseline Trip Generation Comparison

	Rate			AM	Peak Ho	ur_	<u>PM</u>	Peak Ho	ur_	SAT	Γ Peak Ho	ur
Development/Land Use	Source	Size	Units	In	Out	Total	In	Out	Total	In	Out	Total
Approved Conditions												
Existing Building (Demolished))											
Research and Development (1)	ITE (760)	105,000	GFA	113	23	136	21	117	138	17	12	29
Proposed Conditions												
New Buildings/Uses												
General Office (2)	LATR	10,165	GFA	13	2	15	4	19	23	3	2	5
Retail (Without Major Grocer) (3)	<u>LATR</u>	102,535	GLA	100	91	191	396	365	761	462	427	889
Total Proposed Site Trips				113	93	206	400	384	784	465	429	894
Net Total Site Trips (Proposed	vs. Approve	ed)		-	70	70	379	267	646	448	417	865

Notes: (1) Trip generation based on rates and equations in the Institute of Transportation Engineers' (ITE) <u>Trip Generation Manual</u>, Ninth Edition.

⁽²⁾ Weekday AM and PM calculations based on the LATR equations for office uses. ITE equations were used for Saturday calculations.

⁽³⁾ Weekday AM and PM calculations based on the LATR equations for retail uses without a "Major Grocer". ITE equations were used for Saturday calculations.

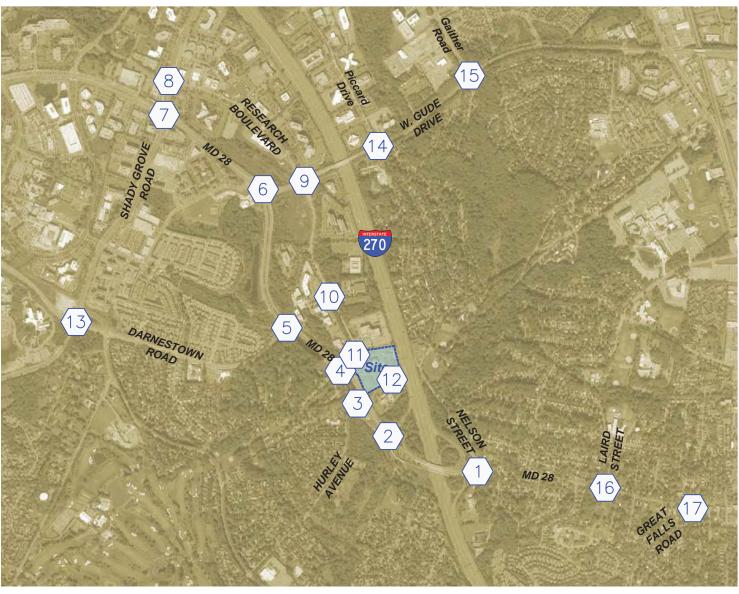
WELLS + ASSOCIATES

When comparing the baseline trip generation estimates for the proposed new retail and office uses to the estimates for the recently demolished 105,000 SF of R&D uses, the proposed new uses would generate approximately 70 total net additional weekday AM peak hour trips, 646 total net additional weekday PM peak hour trips, and 865 total net additional mid-day peak hour trips on a Saturday. It is noted that, since the existing 73,000 SF office building will remain with the proposed plan and it was occupied at the time traffic count data was collected, the trips associated with that building were not included when calculating the impact of the site redevelopment. Based on these estimates, the corresponding mitigation requirements for this project are identified on Figure 1-4 (Table 3 from the City's CTR Guidelines) and include the following:

Mitigation Requirements: Projects Generating 350+ Total Additional Site Trips

- > Transportation Improvement Fee
- > Intersection Mitigation if exceeds Intersection Impact Thresholds
- > Trip Reduction Plan consistent with Trip Reduction Tool for Office Uses
- > Transportation Improvement Contribution Consistent With Multimodal Analysis

As previously noted and as outlined in the March 21, 2011 CTR Guidelines, the above estimates were used to identify the base Mitigation Requirements for the project, with the peak hour trips to be adjusted to include pass-by trip reductions for retail uses and reductions for existing uses to be removed in order to identify any specific intersection mitigation (if any) that would be required as a result of the proposed redevelopment.



- W. Montgomery Ave. (MD 28) / I-270 Off-Ramp / Nelson St
- W. Montgomery Ave. (MD 28) / I-270 SB Ramps
- W. Montgomery Ave. (MD 28) / Hurley Avenue
- W. Montgomery Ave. (MD 28) / Research Blvd / Crofton Lane
- W. Montgomery Ave. (MD 28) /
 Darnestown Road

- 6 W. Montgomery Ave. (MD 28) / West Gude Drive
- W. Montgomery Ave. (MD 28) / Shady Grove Road
- 8 Research Blvd / Shady Grove Rd
- 9 Research Blvd / W. Gude Dr.
- Research Blvd / 1600 Research Blvd Driveway
- 11 Research Blvd / Site Driveway

12 Access Drive / Site Driveway

- \$\langle 13 \rangle Shady Grove Rd / Darnestown Rd
- 14 W Gude Dr / Piccard Dr
- 15 W Gude Dr / Gaither Rd
- W. Montgomery Ave. (MD 28) / Laird St
- W. Montgomery Ave. (MD 28) / Great Falls Rd



Figure 2-1 CTR Study Intersections

Research Row

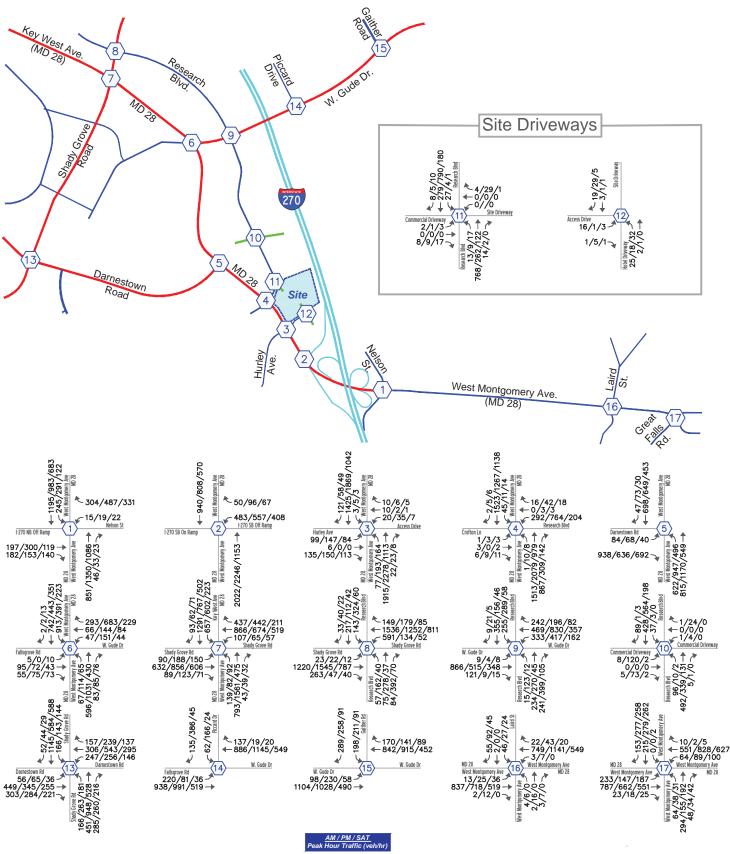


Figure 2-2 Baseline 2014 Weekday AM and PM & Saturday Mid-Day Peak Hour Volumes Research Row CTR



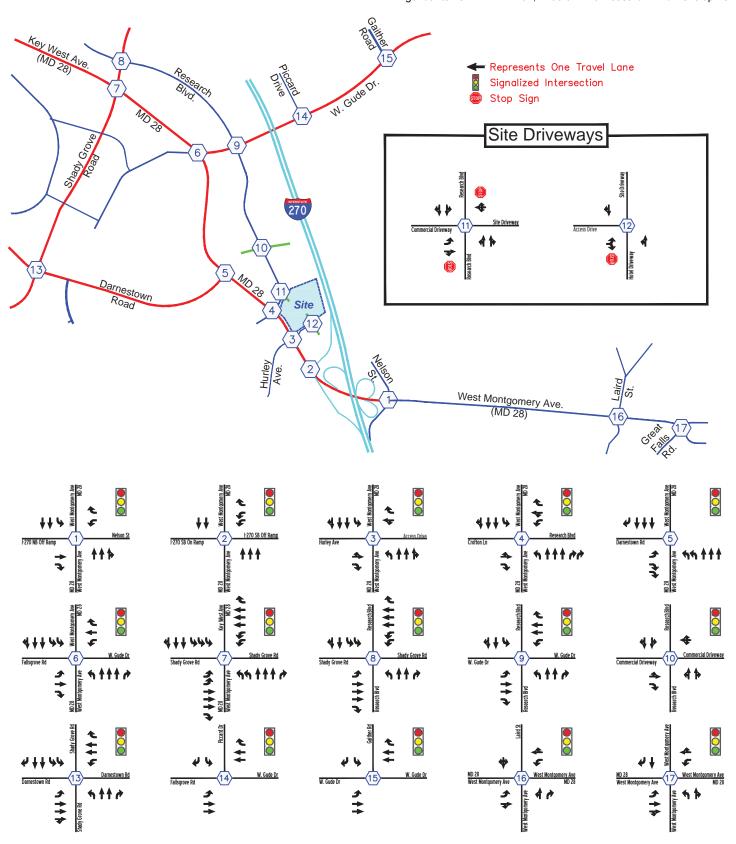


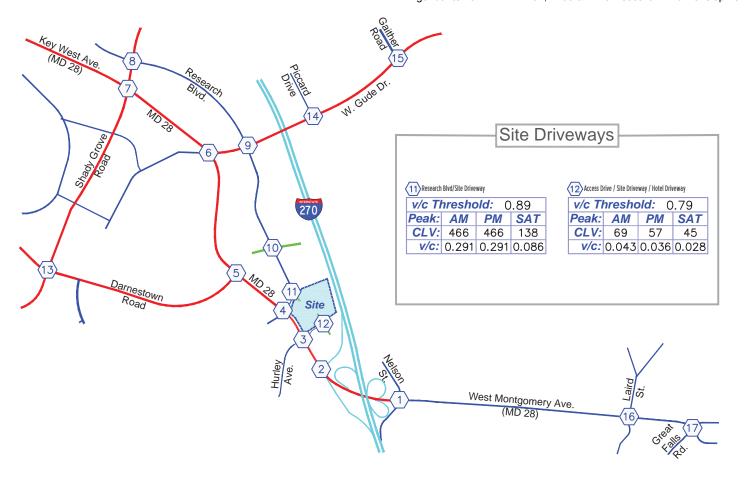
Figure 3-1
Existing Lane Use and Traffic Controls at Study Intersection
Research Row CTR



Table 3-1
Capacity Thresholds for Study Area Intersections

Intersection	Signal Phases		Cycle Length			ongesti		V/C Ratio Threshold	Intersection 10. Research Blvd / 1600		Signal Phases Cycle Length			Congestion Standard			V/C Ratio Threshold
01. W. Montgomery Ave (MD 28) / Nelson Ave Signalized Control	4	120	PM 120	SAT 110	AM 1500	PM 1500	SAT 1400	0.99	10. Research Blvd / 1600 Research Driveway Signalized Control	2	60	PM 60	SAT 60	AM 150	PM 1500	SAT 1500	0.89
02. W. Montgomery Ave (MD 28) / I-270 SB Ramps Signalized Control	2	120	PM 120	SAT 110	AM 1650	PM 1650	SAT 1600	0.99	11. Research Blvd / Site Driveway Two-Way Stop Control	2	90	PM 90	SAT 90	AM 1600	PM 1600	SAT 1600	0.89
03. W. Montgomery Ave (MD 28) / Hurley Ave Signalized Control	3	120	PM 120	SAT 110	AM 1600	PM 1600	SAT 1500	0.89	12. Access Drive / Site Driveway Two-Way Stop Control	2	90	PM 90	SAT 90	AM 1600	PM 1600	SAT 1600	0.79
04. W. Montgomery Ave (MD 28) / Research Blvd Signalized Control	4	AM 120	PM 120	SAT 110	AM 1500	PM 1500	SAT 1400	0.89	13. Shady Grove Rd / Darnestown Rd Signalized Control	4	AM 120	PM 120	SAT 120	AM 1500	PM 1500	SAT 1500	0.89
05. W. Montgomery Ave (MD 28) / Darnestown Rd Signalized Control	3	AM 120	PM 120	SAT 110	AM 1600	PM 1600	SAT 1500	0.89	14. West Gude Dr / Piccard Dr Side-Street Stop Controlled	3	100	PM 100	SAT 90	AM 1500	PM 1500	SAT 1500	0.89
06. W. Montgomery Ave (MD 28) / Gude Dr Signalized Control	3	AM 150	PM 150	SAT 150	AM 1650	PM 1650	SAT 1650	0.99	15. West Gude Dr / Gaither Rd Side-Street Stop Controlled	3	AM 100	PM 100	SAT 100	AM 1500	PM 1500	SAT 1500	0.89
07. W. Montgomery/Key West Ave (MD 28) / Shady Grove Rd Signalized Control	4	AM 150	PM 150	SAT 150	AM 1550	PM 1550	SAT 1550	0.99	16. W. Montgomery Ave (MD 28) / Laird St Side-Street Stop Controlled	2	120	PM 120	SAT 90	AM 1650	PM 1650	SAT 1600	0.89
08. Research Blvd / Shady Grove Rd Signalized Control	4	AM 150	PM 150	SAT 150	AM 1550	PM 1550	SAT 1550	0.89	17. W. Montgomery Ave (MD 28) / Great Falls Rd Side-Street Stop Controlled	3	90	PM 120	SAT 90	AM 1500	PM 1600	SAT 1500	0.99
09. Research Blvd / Gude Dr Signalized Control	4	AM 120	PM 120	SAT 90	AM 1500	PM 1500	SAT 1400	0.89									

	Intersection Capa	city (100% of capacity)	
Cycle Length		Number of Phases	
(seconds)	2	3	4 or more
0 - 89	1500	1400	1300
90 - 119	1600	1500	1400
120 - 149	1650	1600	1500
150 - 999	1700	1650	1550



1 W. Montgomery Ave (MD 28)/I-270 NB Ramp/Nelson St v/c Threshold: 0.99 Peak: AM PM SAT **CLV:** 867 1301 881

v/c: 0.578 0.867 0.629

2 West Montgomery Ave (MD 28)/I-270 SB Ramps v/c Threshold: 0.99 Peak: AM PM SAT *CLV:* 1038 | 1165 | 672 v/c: 0.629 0.706 0.420 3 West Montgomery Ave (MD 28)/Hurley Ave v/c Threshold: 0.89 Peak: AM PM SAT **CLV:** 861 1128 685 v/c: 0.538 0.705 0.457 4 West Montgomery Ave (MD 28)/Research Blvd v/c Threshold: 0.89 Peak: AM PM SAT CLV: 790 1244 562 v/c: 0.527 0.829 0.401

5 West Montgomery Ave (MD 28)/Darnestown Rd v/c Threshold: 0.89 Peak: AM PM SAT **CLV:** 755 883 535 v/c: 0.472 0.552 0.357

6 West Montgomery Ave (MD 28)/W. Gude Dr v/c Threshold: 0.99 SAT Peak: AM PM

854

v/c: 0.555 0.518 0.235

388

7 West Montgomery Ave (MD 28)/Shady Grove Rd v/c Threshold: 0.99 Peak: AM PM SAT 1181 549 **CLV**: 969 v/c: 0.625 0.762 0.354 8 Research Blvd/Shady Grove Rd v/c Threshold: 0.89 Peak: AM PM SAT *CLV:* 1002 | 1158 | 400 v/c: 0.646 0.747 0.258 9 Research Blvd/West Gude Dr v/c Threshold: 0.89 Peak: AM PM SAT **CLV:** 1230 | 1193 450 v/c: 0.820 0.795 0.321 (10) Research Blvd/1600 Research Blvd Driveway v/c Threshold: 0.89 Peak: AM PM SAT **CLV:** 431 475 112 v/c: 0.287 0.317 0.075

13 Shady Grove Rd/Darnestown Rd

CLV: 916

v/c Threshold: 0.89 SAT Peak: AM PM **CLV:** 1365 | 1165 893 v/c: 0.910 0.777 0.595 14 West Gude Dr/Piccard Dr Peak: AM

v/c Threshold: 0.89 PM | SAT CLV: 780 993 357 v/c: 0.520 0.662 0.238 15 West Gude Dr/Gaither Rd v/c Threshold: 0.89 PM SAT Peak: AM **CLV**: 803 970 404 v/c: 0.535 0.647 0.269 16 West Montgomery Ave (MD 28)/Laird St v/c Threshold: 0.89 Peak: AM PM SAT **CLV:** 954 1341 681 v/c: 0.578 0.813 0.426 (17) West Montgomery Ave (MD 28)/Great Falls Rd v/c Threshold: 0.99 Peak: AM PM SAT CLV: 1159 | 1313 | 1134 v/c: 0.724 0.821 0.756

Figure 3-4 Baseline 2014 CLV & v/c



COMPONENT 4

2019 BACKGROUND FUTURE CONDITIONS (WITHOUT THE PROPOSED REDEVELOPMENT)

4.1 Overview

This section provides an evaluation of the forecasted 2019 background future traffic conditions at each of the 17 study intersections without the proposed redevelopment of the site. One (1) pipeline project was identified by City of Rockville Staff for inclusion in the forecasts and analyses.

4.2 Pipeline Projects:

The pipeline development included in this study is listed on the scoping letter issued by the City of Rockville (see Appendix A) and is detailed below. The locations of each pipeline development component are identified on Figure 4-1.

Fallsgrove:

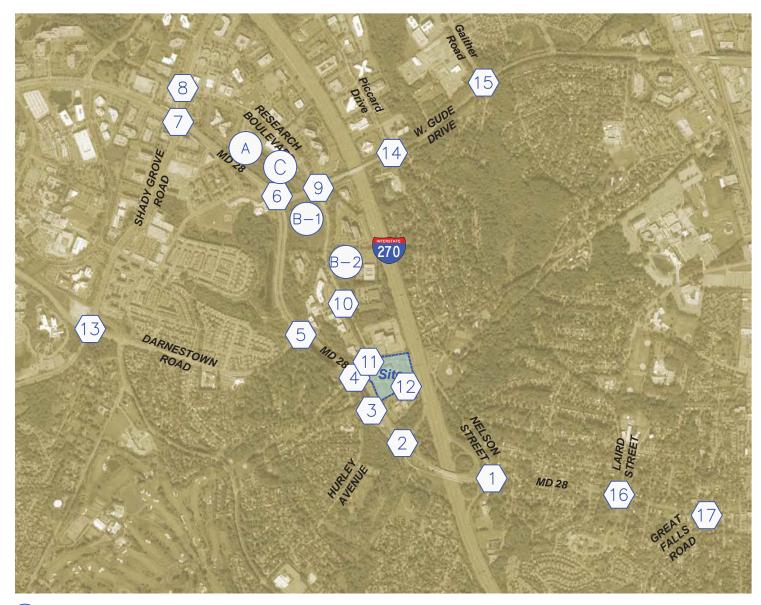
- (A) 90,308 SF of Medical Office (East of MD 28 & North of W. Gude Dr.)
- (B-1) 212,844 SF of General Office (Key West Center at Fallsgrove)
- (B-2) 180,000 SF of General Office (1700 Research Boulevard)
- (C) 80,000 SF of General Office (West of Research Blvd. & North of W. Gude Dr.)

4.3 Pipeline Projects Trip Generation

Trip generation estimates for each Pipeline project component were prepared as directed in the City's CTR Guidelines. A summary of the trip generation for each pipeline development is provided in Table 4-1 below and indicates that the pipeline development would generate a combined total of 996 AM and 1,009 PM peak hour trips on a weekday, and 533 mid-day peak hour trips on Saturday. The combined traffic assignments for all pipeline development components are summarized on Figure 4-2(D), and approved individual pipeline development component traffic assignments are provided on Figures 4-2(A), 4-2(B-1), 4-2(B-2), and 4-2(C) and in Appendix F.

Table 4-1
Pipeline Development Trip Generation

			<u>AM</u>	Peak Ho	<u>ur</u>	<u>PM</u>	Peak Ho	<u>ur</u>	SAT	' Peak Ho	<u>ur</u>
Development/Land Use	Size	Units	In	Out	Total	In	Out	Total	In	Out	Total
1. Fallsgrove											
(A) Medical Office (East of Shady Grove Rd. & North of W. Gude Dr.)	90,308	3 SF	171	45	216	74	192	266	187	141	328
(B-1) General Office (Key West Center at Fallsgrove)	212,844	4 SF	308	46	354	56	271	327	50	42	92
(B-2) General Office (1711 Research Boulevard)	180,000) SF	259	39	298	48	232	280	42	36	78
(C) General Office (West of Research Blvd. & North of W. Gude Dr.)	<u>80,000</u>	<u> SF</u>	111	17	128	23	113	136	19	16	<u>35</u>
Subtotal - Fallsgrove Pipeline Trips	563,15	2 SF	849	147	996	201	808	1,009	298	235	533
Total Pipeline Development Trips			849	147	996	201	808	1,009	298	235	533



A Fallsgrove - 90,308 SF of Medical Office

ig(eta-1ig) Fallsgrove - 212,844 SF of Office

 $oxed{eta-2}$ Fallsgrove - 180,000 SF of Office

C Fallsgrove - 80,000 SF of Office

Figure 4-1
Pipeline Locations
Research Row



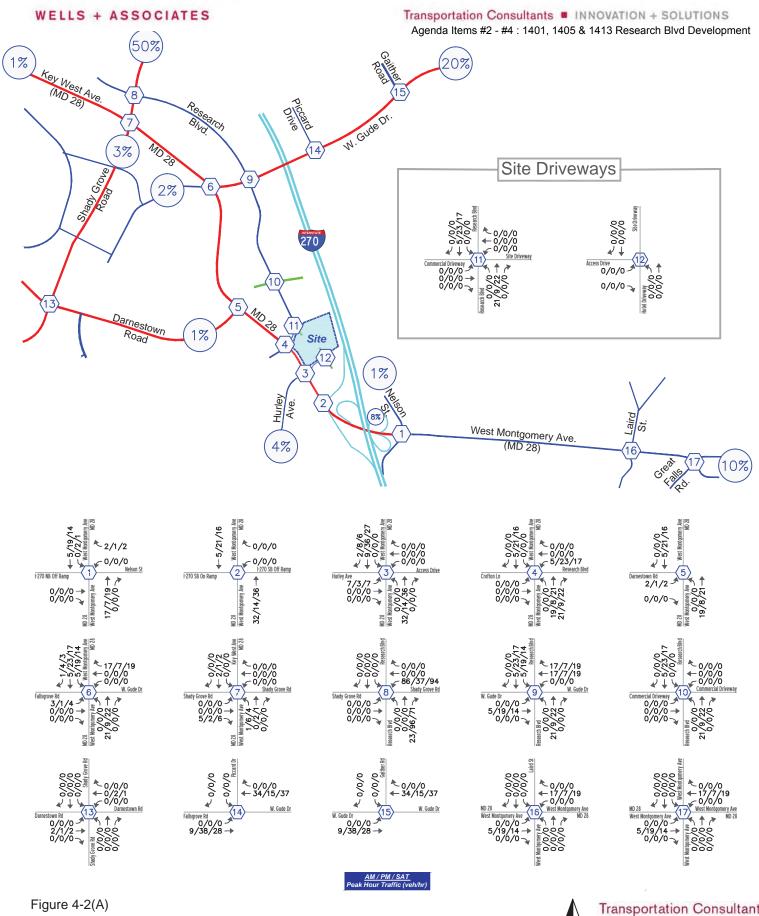


Figure 4-2(A)
Fallsgrove - 90,308 SF of Medical Office
Research Row CTR



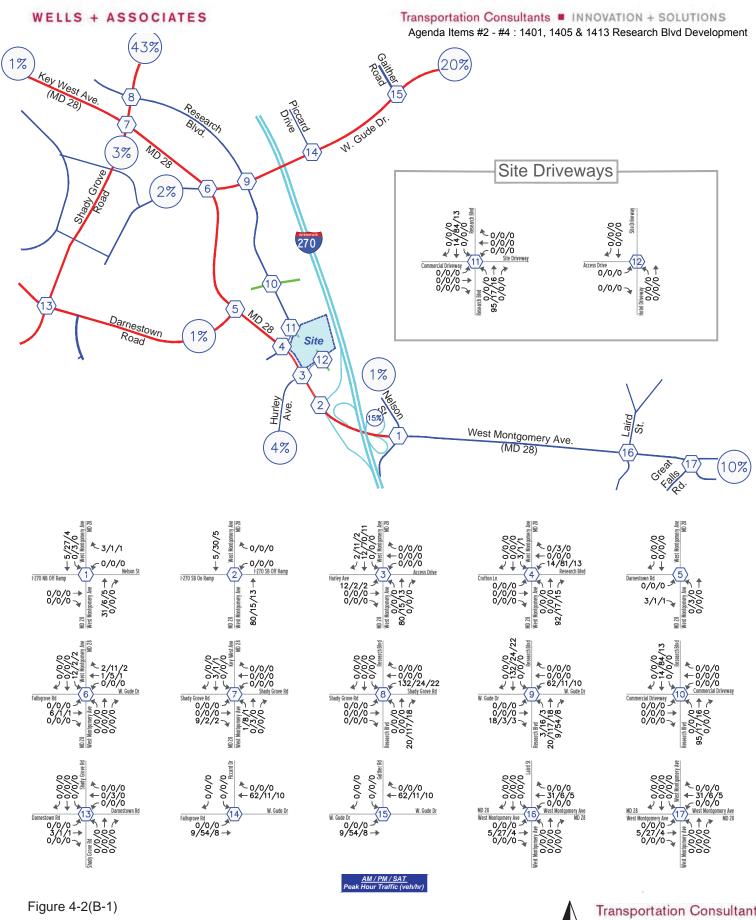


Figure 4-2(B-1)
Fallsgrove - 212,844 SF of Office
Research Row CTR

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NORTH

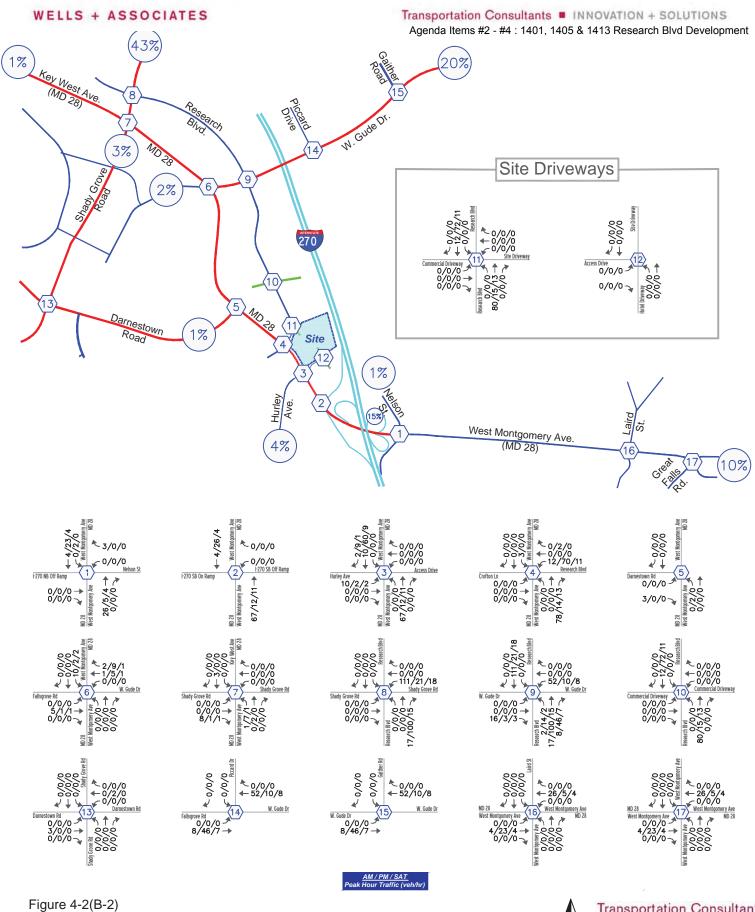


Figure 4-2(B-2)
Fallsgrove - 180,000 SF of Office
Research Row CTR
City of Rockville, Maryland

Transportation Consultants
INNOVATION + SOLUTIONS
NORTH

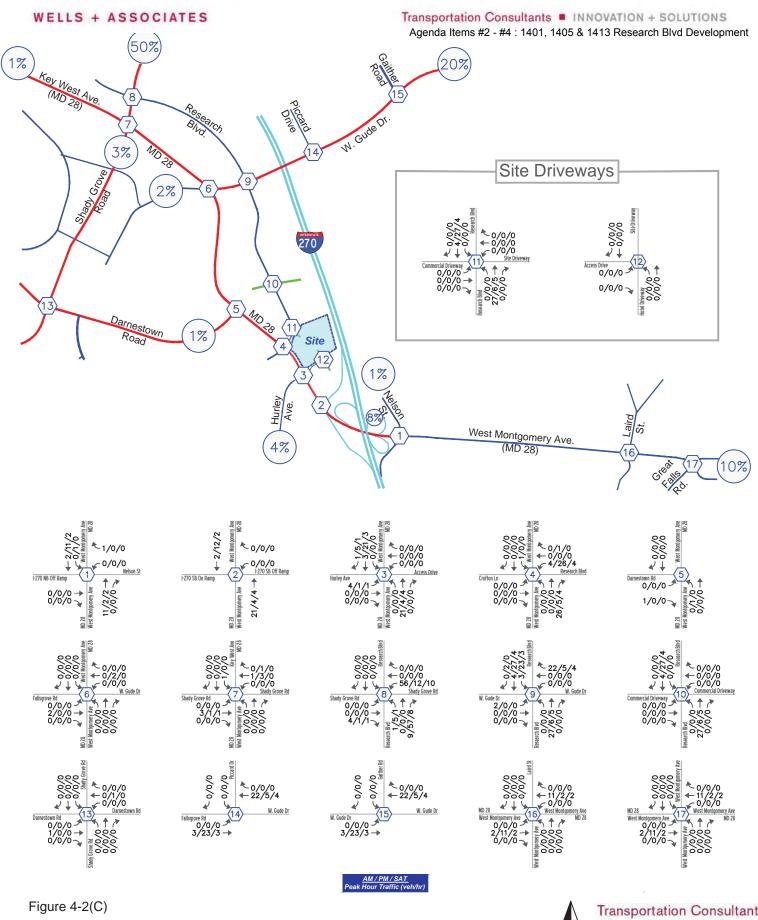


Figure 4-2(C)
Fallsgrove - 80,000 SF of Office
Research Row CTR
City of Rockville, Maryland

Transportation Consultants
INNOVATION + SOLUTIONS
NORTH

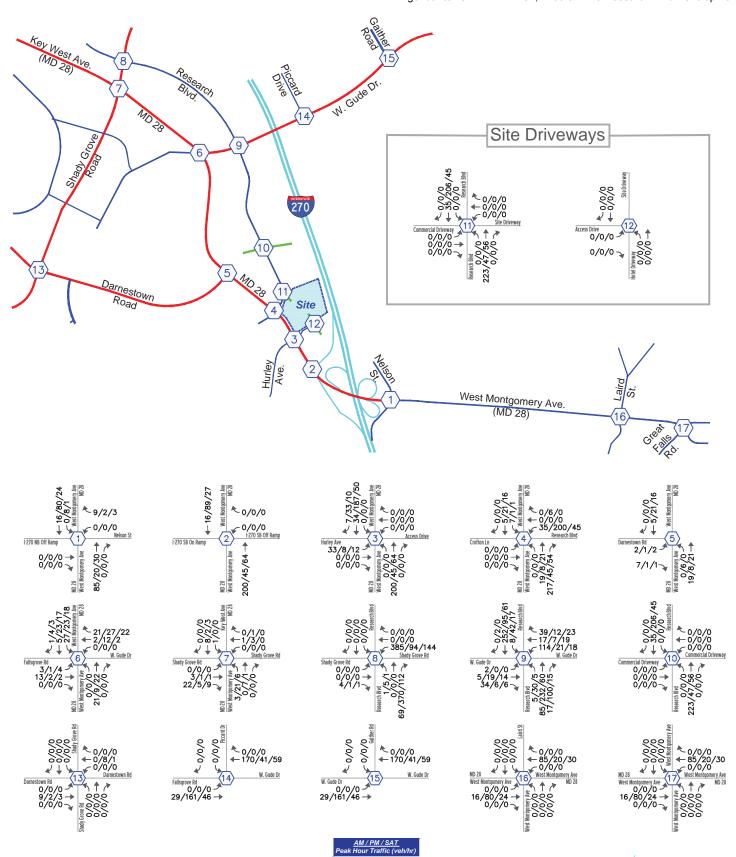


Figure 4-2(D)
Combined Total Traffic For All Pipeline Developments



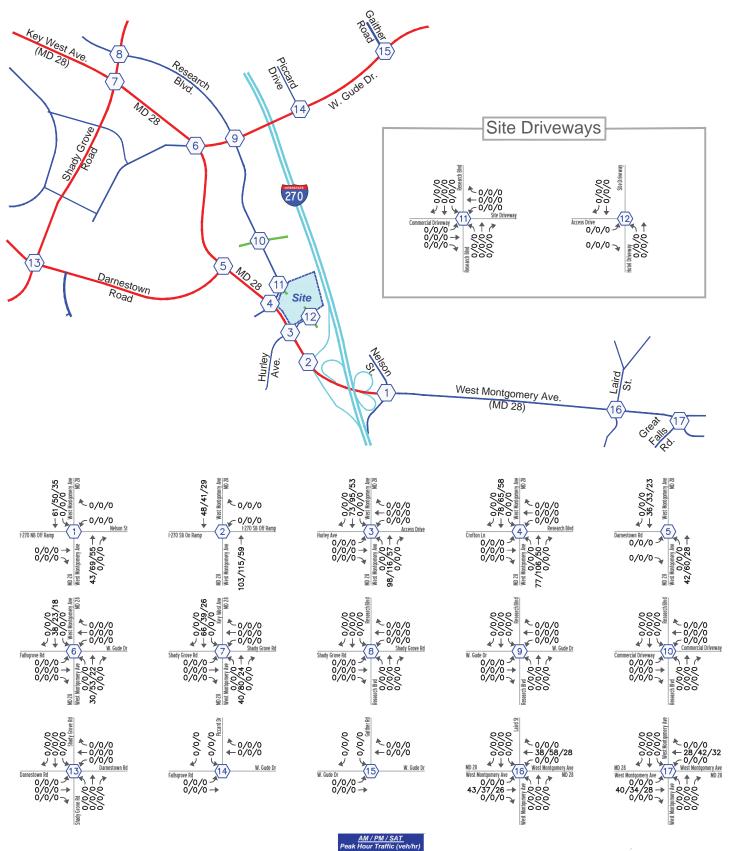


Figure 4-3
Regional Growth in Through Traffic Along MD 28 (1% Annual from 2014-2019)



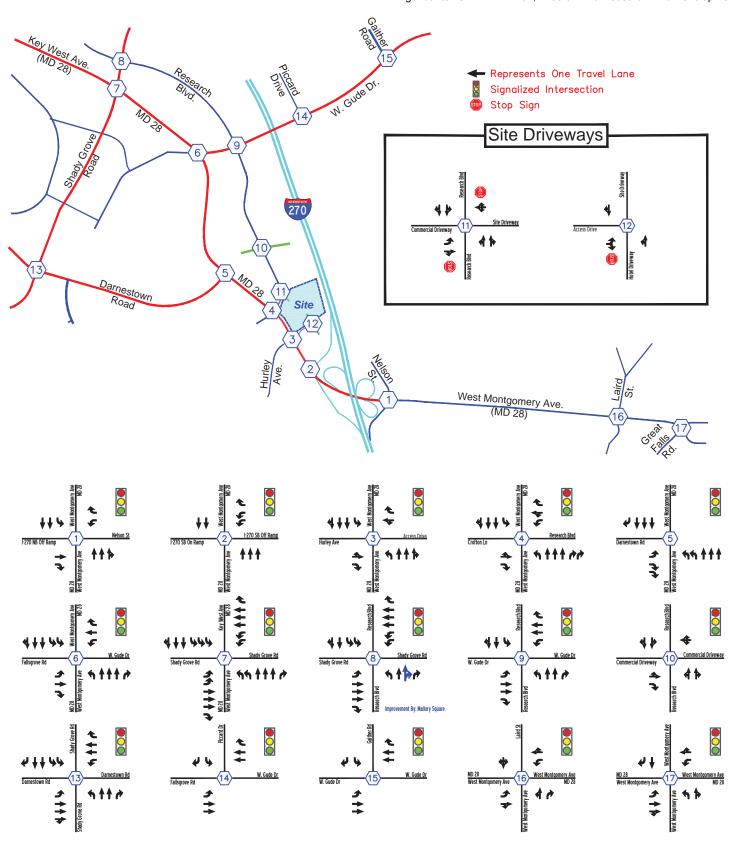


Figure 4-5
Background Future Lane Use and Traffic Controls
Research Row CTR



Table 4-2 Background Critical Lane Volume (CLV) and Volume-To-Capacity (v/c) Summary

background Critica				vjanu	volullie	-то-сар		ckground Fut	
	Congestion Standard	Threshold	Measure of Performance	В	aseline Existir	ng		(Year 2019)	
Intersection	(100% Capacity)	× ×	Me. Perf	AM	PM	SAT	AM	PM	SAT
01. W. Montgomery Ave (MD 28) / Nelson Ave	AM PM SAT 1500 1500 1400	v/c 0.99	CLV	867	01. W. Mont 1301	gomery Ave (I 881	MD 28) / Nels 920	on Ave 1337	917
Nelson Ave	Signalized	(max)	v/c	0.578	0.867	0.629	0.613	0.891	0.655
	Intersection		LOS	А	D	В	В	D	В
02. W. Montgomery Ave (MD 28) / I-	AM PM SAT	v/c			I 02. W. Montgo	mery Ave (MI	IL D 28) / I-270 S	B Ramps	
270 SB Ramps	1650 1650 1600	0.99	CLV	1038	1165	672	1173	1229	720
	Signalized	(max)	v/c	0.629	0.706	0.42	0.711	0.745	0.45
	Intersection		LOS	В	С	А	С	С	А
03. W. Montgomery Ave (MD 28) / Hurley Ave	AM PM SAT 1600 1600 1500	v/c 0.89	CLV	861	03. W. Mont 1128	gomery Ave (685	MD 28) / Hurl 1072	ey Ave 1357	751
nuncy Ave	Signalized	(max)	v/c	0.538	0.705	0.457	0.67	0.848	0.501
	Intersection		LOS	А	С	А	В	D	А
04. W. Montgomery Ave (MD 28) /	AM PM SAT	v/c			I 04. W. Montg	omery Ave (N	I <u>I</u> 1D 28) / Resea	rch Blvd	
Research Blvd	1500 1500 1400	0.89	CLV	790	1244	562	861	1415	617
	Signalized	(max)	v/c	0.527	0.829	0.401	0.574	0.943	0.441
	Intersection		LOS	А	D	А	Α	E	Α
05. W. Montgomery Ave (MD 28) /	AM PM SAT	v/c				omery Ave (M			550
Darnestown Rd	1600 1600 1500	0.89 (max)	CLV v/c	755 0.472	883 0.552	535 0.357	783 0.489	915 0.572	550 0.367
	Signalized Intersection	, ,	LOS	A	A	A	А	A	A
06. W. Montgomery Ave (MD 28) /	AM PM SAT	v/c				ntgomery Ave			
Gude Dr	1650 1650 1650	0.99	CLV	916	854	388	964	895	422
	Signalized	(max)	v/c	0.555	0.518	0.235	0.584	0.542	0.256
	Intersection		LOS	А	А	А	Α	Α	А
07. W. Montgomery/Key West Ave	AM PM SAT	v/c						hady Grove Rd	
(MD 28) / Shady Grove Rd	1550 1550 1550	0.99	CLV v/c	969 0.625	1181 0.762	549 0.354	1002 0.646	1216 0.785	564 0.364
	Signalized	(max)	LOS	0.623 B	0.762 C		0.040 B	0.783 C	
	Intersection		LUS	В		Α			А
08. Research Blvd / Shady Grove Rd	AM PM SAT 1550 1550 1550	v/c 0.89	CLV	1002	08. Res	search Blvd / S 400	1235	1289	526
	Signalized	(max)	v/c	0.646	0.747	0.258	0.797	0.832	0.339
	Intersection		LOS	В	С	А	С	D	А
09. Research Blvd / Gude Dr	AM PM SAT	v/c			09.	. Research Blvo	d / Gude Dr		
	1500 1500 1400	0.89	CLV	1230	1193	450	1415	1397	530
	Signalized	(max)	v/c	0.820	0.795	0.321	0.943	0.931	0.379
	Intersection		LOS	D	С	А	E	E	А
10. Research Blvd / 1600 Research Driveway	AM PM SAT 1500 1500 1500	v/c 0.89	CLV	431	10. Researc	th Blvd / 1600	Research Driv	reway 584	136
Direcway	Signalized	(max)	v/c	0.287	0.317	0.075	0.358	0.389	0.091
	Intersection		LOS	А	Α	А	А	А	А
11. Research Blvd / Site Driveway	AM PM SAT	v/c			11. Re	esearch Blvd /	IL	1	
,	1600 1600 1600	0.89	CLV	466	466	138	594	582	162
	Two-Way Stop	(max)	v/c	0.291	0.291	0.086	0.371	0.364	0.101
	Control		LOS	А	А	А	А	Α	Α
12. Access Drive / Site Driveway	AM PM SAT 1600 1600 1600	v/c 0.79	CLV	69	12. A	ccess Drive / S	203	189	73
	Two-Way Stop	0.79 (max)	v/c	0.043	0.036	0.028	0.127	0.118	0.046
	Control		LOS	А	А	А	А	А	А
13. Shady Grove Rd / Darnestown	AM PM SAT	v/c			13. Sha	dy Grove Rd /	Darnestown F	l ₹d	
Rd	1500 1500 1500	0.89	CLV	1365	1165	893	1366	1171	894
	Signalized	(max)	v/c	0.910	0.777	0.595	0.911	0.781	0.596
	Intersection		LOS	Е	С	А	E	С	Α
14. West Gude Dr / Piccard Dr	AM PM SAT	v/c	CLV	700	993	West Gude Dr		1015	389
	1500 1500 1500 Signalized	0.89 (max)	V/c	780 0.520	0.662	357 0.238	871 0.581	0.677	0.259
	Intersection		LOS	А	В	А	А	В	А
15. West Gude Dr / Gaither Rd	AM PM SAT	v/c				West Gude Dr			
, Gardier na	1500 1500 1500	0.89	CLV	803	970	404	864	992	435
	Signalized	(max)	v/c	0.535	0.647	0.269	0.576	0.661	0.29
	Intersection		LOS	А	В	А	Α	В	Α
16. W. Montgomery Ave (MD 28) /	AM PM SAT	v/c				ntgomery Ave			
aird St	1650 1650 1600	0.99 (max)	CLV v/c	954 0.578	1341 0.813	681 0.426	1043 0.632	1423 0.862	742 0.464
	Signalized Intersection	(ax)	LOS	0.578 A	D.813	A A	B	D	A
17 M Mantagana - 1 - (840 00) (LUS						
17. W. Montgomery Ave (MD 28) / Great Falls Rd	AM PM SAT 1500 1600 1500	v/c 0.99	CLV	1159	17. W. Montg	omery Ave (N 1134	1295	1379	1199
	Signalized	(max)	v/c	0.724	0.821	0.756	0.809	0.862	0.799
	Intersection		LOS	С	D	С	D	D	С

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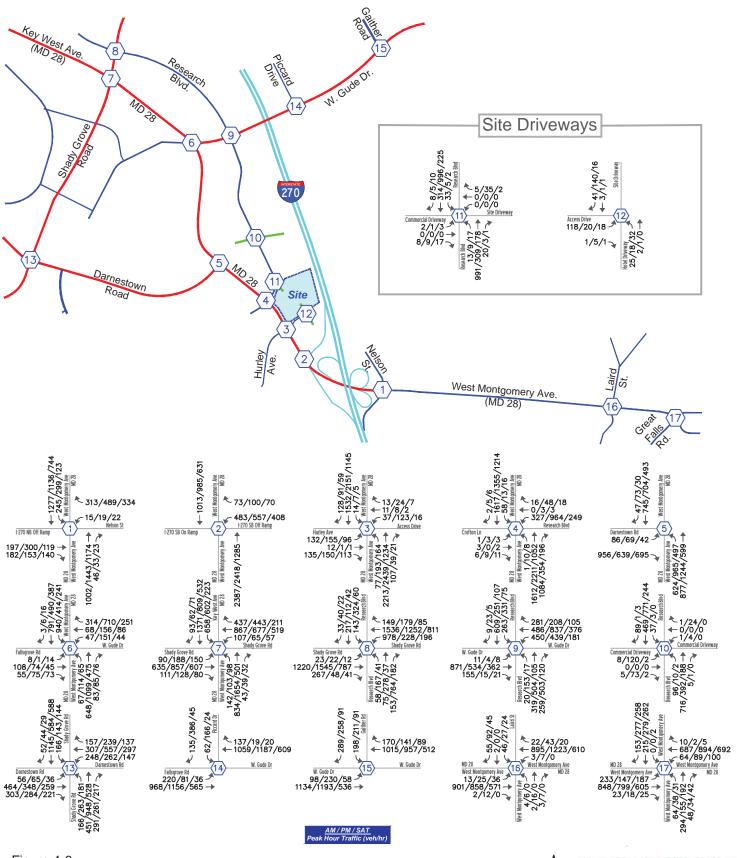
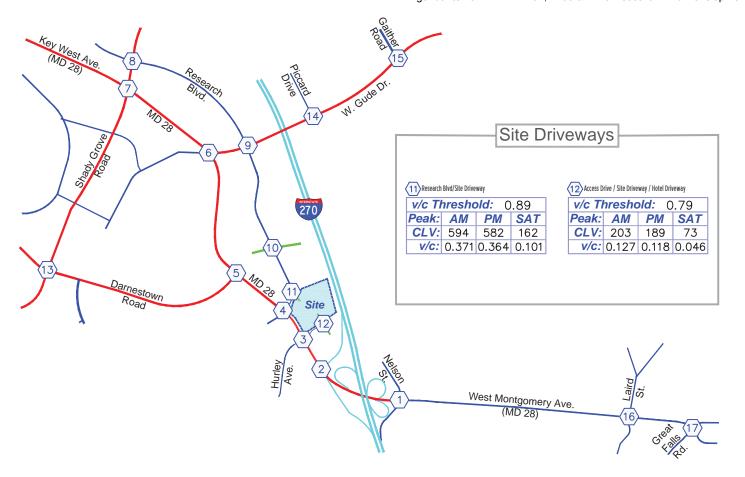


Figure 4-6 2019 Background Future Traffic Forecasts (No-Build)





1) W. Montgomery Ave (MD 28)/I-270 NB Ramp/Nelson St V/c Threshold: 0.99 Peak: AM PM SAT CLV: 920 1337 917 V/c: 0.613 0.891 0.655 2) West Montgomery Ave (MD 28)/L-270 SB Ramps

v/c Threshold: 0.99

Peak: AM PM SAT

CLV: 1173 1229 720

v/c: 0.711 0.745 0.450

3) West Montgomery Ave (MD 28)/Hurley Ave

V/c Threshold: 0.89

Peak: AM PM SAT

CLV: 1072 1357 751

V/c: 0.670 0.848 0.501

 V/c Threshold:
 0.89

 Peak:
 AM
 PM
 SAT

 CLV:
 861
 1415
 617

 V/c:
 0.574
 0.943
 0.441

| V/c Threshold: 0.89 | Peak: AM | PM | SAT | CLV: 783 | 915 | 550 | V/c: 0.489 | 0.572 | 0.367 |

(a) West Montgomery Ave (MD 28)/W. Gude Dr | V/C Threshold: 0.99 | Peak: AM PM SAT | CLV: 964 895 422 | V/C: 0.584 0.542 0.256 | V/c Threshold: 0.99 | Peak: AM | PM | SAT | CLV: 1002 | 1216 | 564 | V/c: 0.646 | 0.785 | 0.364 |

8) Research Blvd/Shady Grove Rd

V/c Threshold: 0.89

Peak: AM PM SAT

CLV: 1235 1289 526

V/c: 0.797 0.832 0.339

9) Research Blwd/West Guide Dr

| V/C Threshold: 0.89 |
Peak: AM	PM	SAT
CLV: 1415	1397	530
V/C: 0.943	0.931	0.379

\(\begin{align*} \begin{align*} \lorentering \text{Research Blvd/l600 Research Blvd Driveway} \) \(\begin{align*} \begin{ali

13) Shady Grove Rd/Damestown Rd

v/c Threshold: 0.89

Peak: AM PM SAT

CLV: 1366 1171 894

v/c: 0.911 0.781 0.596

\[\begin{align*} \text{V/c Threshold:} & 0.89 \\ \begin{align*} \

\(\begin{align*} \limits \text{West Gude Dr/Gaither Rd} \) \(\begin{align*} \be

| V/c Threshold: 0.89 | Peak: AM PM SAT | CLV: 1043 | 1423 | 742 | V/c: 0.632 | 0.862 | 0.464 |

77) West Montgomery Ave (MD 28)/Great Falls Rd

V/C Threshold: 0.99

Peak: AM PM SAT

CLV: 1295 1379 1199

V/C: 0.809 0.862 0.799

Figure 4-7 2019 Background Future CLV & v/c



COMPONENT 5

TRIP GENERATION FOR THE PROPOSED REDEVELOPMENT

5.1 Overview

This section provides estimates of the number of vehicle trips that would be generated by the proposed new retail and office uses, with reductions for retail pass-by trips, and the resulting number of trips that would be added to the area road network after accounting for the vehicle trips that could be generated by the 105,000 SF of R&D uses that were added under background conditions but would not be present with the redevelopment.

5.2 Trips Generated by the Proposed New Retail and Office Uses

Per the City's CTR Guidelines, the number of vehicle trips that would be generated by the new retail and office uses were estimated utilizing the applicable equations from M-NCPPC's LATR Guidelines with pass-by reductions calculated based on the rates and/or equations published in the Institute of Transportation Engineers' *Trip Generation Handbook (limited to 20 percent per the CTR)*. The results are summarized on Table 5-1 and indicate that, at full buildout and occupancy, the redeveloped site would generate approximately 206 weekday AM, 784 weekday PM, and 894 mid-day peak hour trips on Saturday. When accounting for pass-by reductions applicable to the retail trip generation and subtracting the trips for approved but unutilized uses (105,000 SF of R&D), the redevelopment would result in approximately 70 net additional weekday primary AM, 494 net additional weekday primary PM, and 687 net additional mid-day primary Saturday peak hour trips being added to the area road network.

Table 5-1
Site Trip Generation (Proposed vs. Approved)

once trip deneration	(1.10b03	a voi Appi ovcaj									
	Rate		AM	Peak H	<u>our</u>	<u>PM</u>	Peak Ho	<u>our</u>	SAT	' Peak H	<u>our</u>
Development/Land Use	Source	Size Units	In	Out	Total	In	Out	Total	In	Out	Total
Approved Conditions											
Approved (Demolished April	2015)										
Research and Development (1)	ITE (760)	105,000 GFA	113	23	136	21	117	138	17	12	29
Proposed Conditions											
New Buildings/Uses											
General Office (2)	LATR	10,165 GFA	13	2	15	4	19	23	3	2	5
Retail - Primary Trips			100	91	191	320	289	609	373	338	711
Retail - Pass-By Trips (3)	ITE (820)	AM: 0% PM: 20% SAT: 20%				76	76	152	89	89	178
Retail - Total Trips ⁽⁴⁾	LATR (4)	102,535 GLA	100	91	191	396	365	761	462	427	889
Total Primary Trips			113	93	206	324	308	632	376	340	716
<u>Total Pass-By Trips</u>						76	76	152	89	89	178
Total Proposed Site Trips			113	93	206	400	384	784	465	429	894
Net Primary Trips (Proposed vs	. 105,000 SF F	R&D)	-	70	70	303	191	494	359	328	687
Net Pass-By Trips (Proposed vs	. 105,000 SF R	<u>&D)</u>	<u> </u>	_=		<u>76</u>	<u>76</u>	<u>152</u>	89	89	<u>178</u>
Net Total Site Trips (Proposed	vs. 105,000 S	F R&D)	-	70	70	379	267	646	448	417	865

Notes: (1) Trip generation based on rates and equations in the Institute of Transportation Engineers' (ITE) <u>Trip Generation Manual</u>, Ninth Edition.

⁽²⁾ Trip generation based on rates and equations in the M-NCPPC LATR

⁽³⁾ See Section 7.1 for additional information. The Pass-By percentages were identified based on guidance provided in ITE's "Trip Generation Handbook", but were then limited to 20% per the CTR.

⁽⁴⁾ Weekday AM and PM calculations based on the LATR equations for retail uses without a "Major Grocer". ITE equations were used for Saturday calculations.

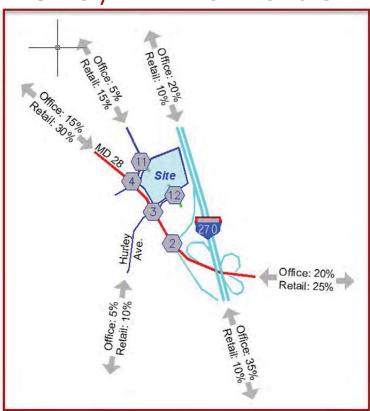
COMPONENT 6 TRIP DISTRIBUTION

6.1 Trip Distribution

The site trip distributions reflect the approved site trip distributions received from City Staff. The approved distributions for retail and office uses are provided in Appendix A, and detailed distributions for all turning movements are provided in the traffic forecasts provided in the Appendix.

A summary of the global distributions, approved by City Staff, are provided below, and detailed traffic forecasts are provided in the Appendix.

OFFICE/RETAIL DISTRIBUTIONS



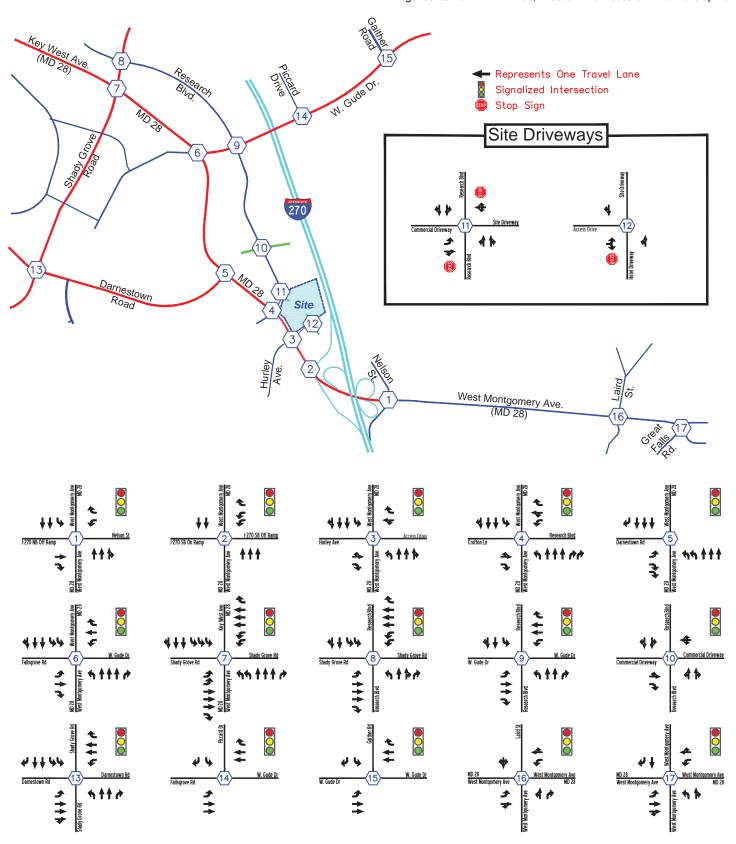
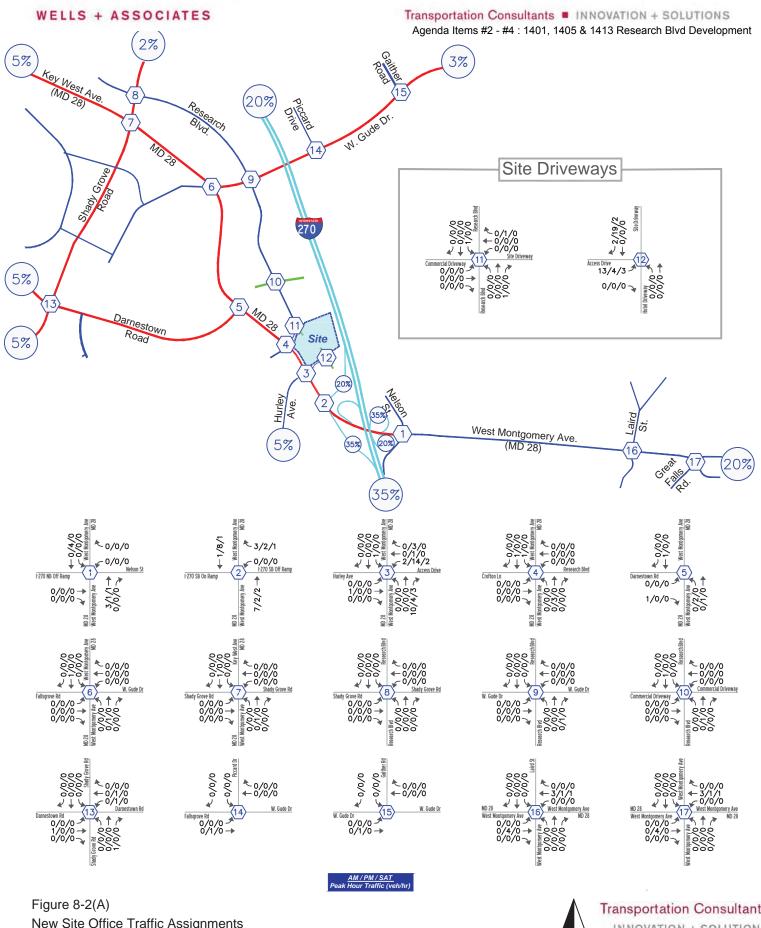


Figure 8-1
Total Future Lane Use and Traffic Controls without Mitigation
Research Row CTR





New Site Office Traffic Assignments Research Row CTR

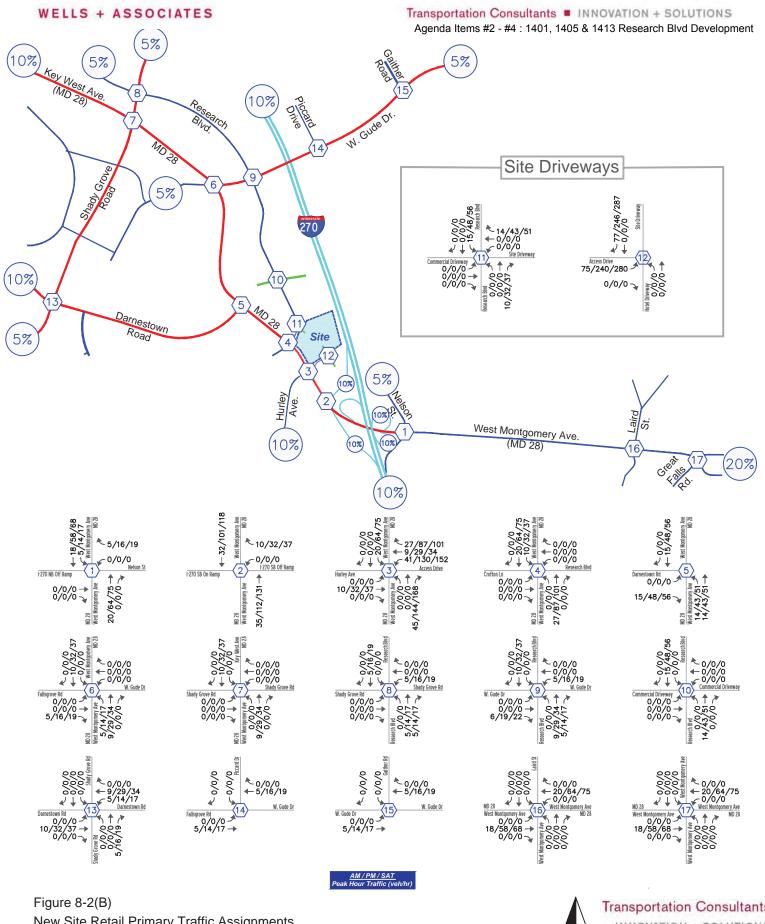


Figure 8-2(B)

New Site Retail Primary Traffic Assignments

Research Row CTR

City of Rockville, Maryland

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NORTH

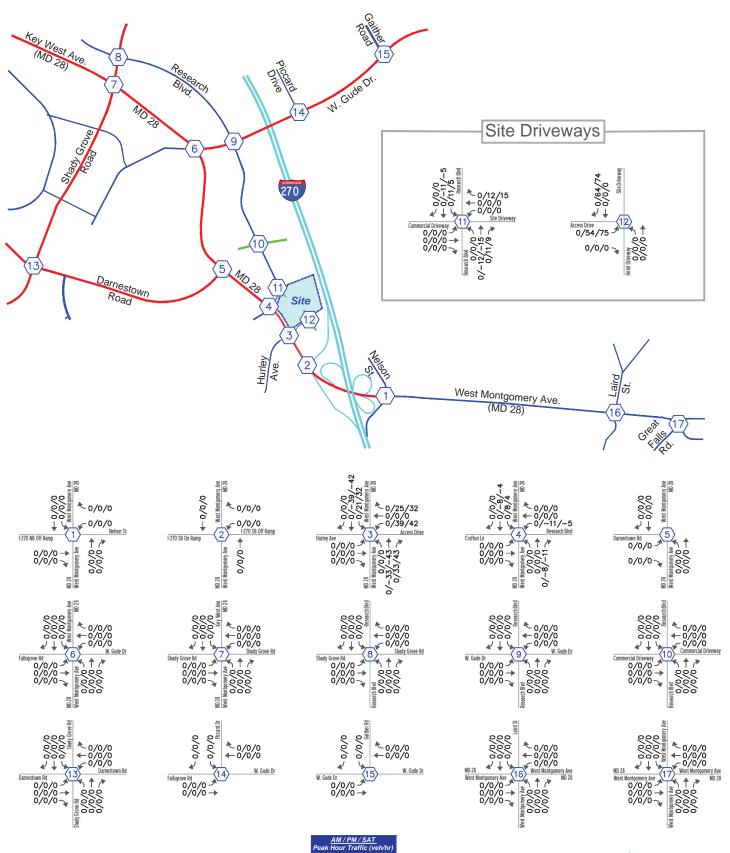


Figure 8-2(C) New Site Retail Pass-By Traffic Assignments



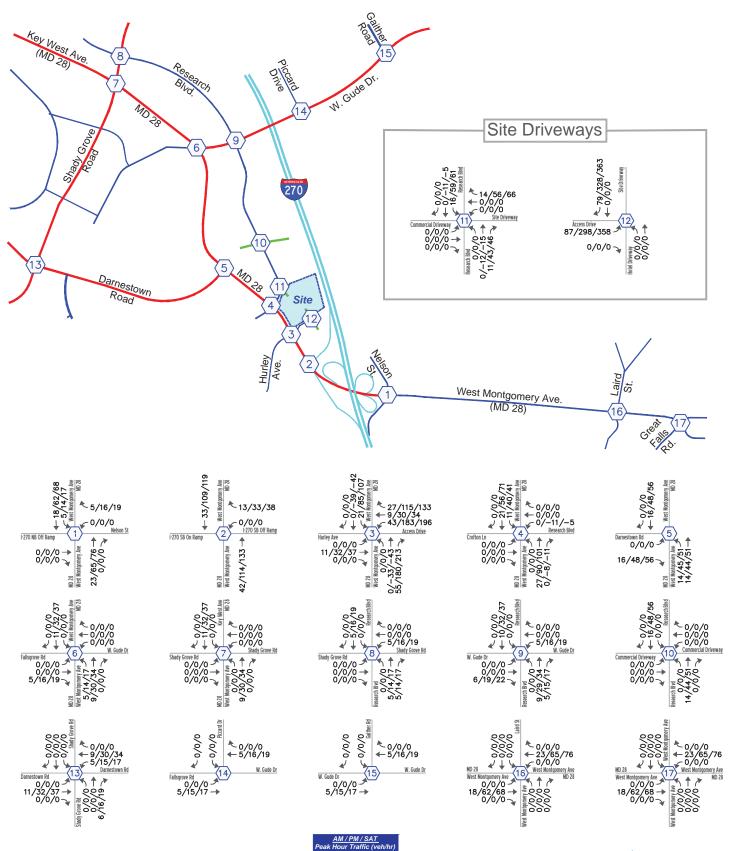


Figure 8-2(D)
Total Traffic Assignments for New Retail and Office Uses (Includes Primary & Pass-By)
Research Row CTR



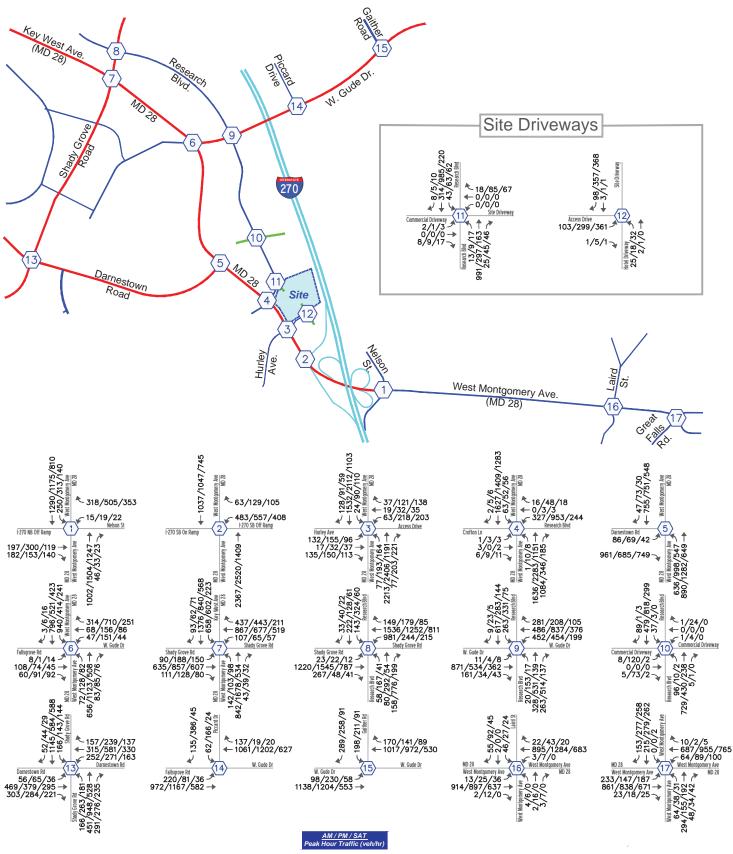


Figure 8-4 2019 Total Future Traffic Forecasts (With Full Buildout of the Proposed Plan)



8.5 2019 Total Future Critical Lane Volume (CLV) Analyses

The 2019 total future peak hour levels of service, critical lane volumes, and volume-to-capacity ratios were evaluated using the City's CLV methodology (Appendix C) based on the total future lane use and traffic controls shown on Figure 8-1 and the 2019 total future traffic projections shown on Figure 8-4.

The level-of-service (LOS) thresholds and intersection capacities and criteria discussed previously (refer to Table 3-1) were utilized in determining the adequacy of the total future CLV estimates. These thresholds and capacities account for the location of each intersection, intersection cycle lengths, and traffic signal phasing.

The results of the total future CLV assessment are summarized in Table 8-1 and Figure 8-5 are discussed below. Detailed CLV worksheets are provided in Appendix H.

A review of the analyses results summarized on Table 8-1 and Figure 8-5 indicate that, consistent with both existing and background future conditions, the Shady Grove Road / Darnestown Road intersection would continue to exceed the acceptable v/c ratio of 0.89 with only a minor increase of 0.002 resulting in a v/c of 0.913 during the AM peak hour.

Consistent with background conditions, the West Montgomery Avenue (MD 28) and West Gude Drive intersections along Research Boulevard are expected to continue to exceed the acceptable v/c ratios of 0.89 during the AM and/or PM peak hours.

The West Montgomery Avenue (MD 28) intersection at Hurley Avenue is forecasted to exceed its acceptable v/c ratio of 0.89 during the PM peak hour with a v/c ratio of 0.942; however, it is noted that the site impact (difference between background and total future) results in an increase to the v/c of less than 0.10 (less than a full ten percent).

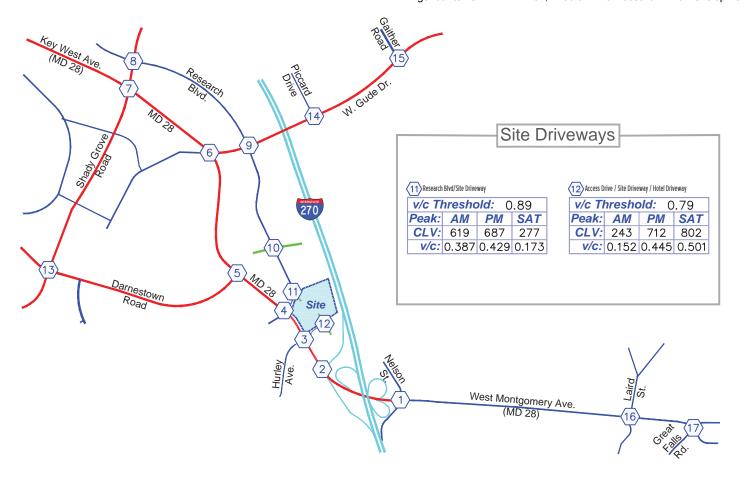
All other intersections would continue to operate at or within the congestion standards under total future conditions during both the weekday AM and PM peak hours and during the mid-day peak hour on Saturday.

The highest CLVs would be realized at the West Gude Drive / Research Boulevard intersection (CLV of 1,422) during the AM peak hour, at the West Montgomery Avenue / Hurley Avenue intersection (CLV of 1,507) during the PM peak hour, and at the West Montgomery Avenue / Great Falls Road intersection during the mid-day on Saturday (CLV of 1,272).

Table 8-1 Total Future Critical Lane Volume (CLV) and Volume-To-Capacity (V/C) Summary

1.1. Moting move yet (No. 20) 1.2. 1.2		Congestion Standard	Standard (Year 2019) (Year 2019)		De	evelopment Imp	<u>pact</u>						
13. M. Montgomery Ave (M0 28)	Intersection		//c T⊦	Mea	AM	PM	SAT	AM	PM	SAT	AM	PM	SAT
Signalized Control C	01. W. Montgomery Ave (MD 28) /		-			1							
20. M. Abdrigomeny Ave (MD 28)	Nelson Ave	1500 1500 1400	4	CLV									
Control Cont		Signalized	(max)	v/c	0.613	0.891	0.655	0.618	0.917	0.688			
1600 1600		Intersection		LOS	В	D	В	В	Е	В			
Signalized Control C	02. W. Montgomery Ave (MD 28) / I-	AM PM SAT	v/c		•		02.\	N. Mon	tgomer	y Ave (N			
W. Montgomery Ave [MD 28] / Interest No. Separate Montgomery Ave [MD 28] / Interest No. Montgomery Ave [MD 28] / Interest No. No	270 SB Ramps	1650 1650 1600	1			i .	ŧ .		1	8			
See Markey Mark		Signalized	(max)	v/c	0.711	0.745	0.45	0.707	0.767	0.479			
Security		Intersection		LOS	С	С	Α	С	С	А			
Sum Management March Management Management	03. W. Montgomery Ave (MD 28) /	AM PM SAT	v/c				03	3. W. M	ontgom	ery Ave	(MD 28) / Hurle	ey Ave	
Separation Color	Hurley Ave	1600 1600 1500	1										
Mathematical Control Mathematical Control		Signalized	(max)	v/c	0.67	0.848	0.501	0.692	0.942	0.673			
Seesarch Bivd Seesarch Biv		Intersection		LOS	В	D	Α	В	E	В			
Signalized Final	04. W. Montgomery Ave (MD 28) /	AM PM SAT	v/c				04.	W. Mor	ntgome	ry Ave (MD 28) / Resear	ch Blvd	
Sum Montgomery Ave (MD 28) A FM FM FM FM FM FM FM	Research Blvd	1500 1500 1400	1	-									
Intersection Six A E A Mitspation Required		Signalized	(max)	v/c	0.574	0.943	0.441	0.583	0.986	0.459			
1600 1500		Intersection		LOS	Α	Е	Α	Α	E	Α			
1600 1500	05. W. Montgomery Ave (MD 28) /	AM PM SAT	v/c				05.1	W. Mon	tgomer	y Ave (N	AD 28) / Darnes	town Rd	
Superalized	Darnestown Rd	1600 1600 1500		CLV	783	915					6		50
Second Bird Form Second Bird Form Second Form		Signalized	(max)	v/c	0.489	0.572	0.367	0.493	0.596	0.400			
X.W. Montgomery Ave (MD 28)		-		LOS	А	А	А	Α	А	А			
Size	06 W Montgomery Ave (MD 28) /	AM PM SAT	v/c)6 W N	l Iontgor	nery Av			пеципец
Signalized Intersection Control	Gude Dr			CLV	964	895							12
Intersection Mol PM SAT Work SAT			(max)	v/c	0.584	0.542	0.256	0.586	0.548	0.263			
27. W. Montgomery/Key West Ave Mo 29 5 5 5 5 5 5 5 5 5				IOS	А	А	Α	А	А	А			
MD 28 / Shady Grove Rd 1550 1550 1550 150 0.09 0.04 0.04 0.04 0.04 0.04 0.04 0.04 0.05 0.008			,				1						Required
Signalized Control C				CIV	1002								13
See Search Blvd Shady Grove Rd AM PM SAT Vc	(IVID 28) / Shady Grove No		1								_	1	-
8. Research Blvd / Shady Grove Rd AM PM SAT Signalized Intersection Sign		-	, ,	100		-		В	_		v/c	≤ 0.99, Site Impact < 1	
1550 1550		ļ		103	В		_ A						Required
Signalized Intersection Control	08. Research Blvd / Shady Grove Rd			CIV	1225	1200	F20						24
10.0 16.0			1										
29. Research Blvd / Gude Dr		_	(IIIax)										
1500 1500 1500 1400 0.88 0.94 0.943 0.931 0.379 0.948 0.951 0.946 0.005 0.020 0.027		Intersection		LOS	С	D	A	С	D	A			
Signalized Control C	09. Research Blvd / Gude Dr	I 1 3	v/c								vd / Gude Dr		
10. Research Blvd / 1600 Research 20. Research Blvd / 1600 Research Blvd /													
10 10 10 10 10 10 10 10		1500 1500 1400	1								0.005		
1500 1500		Signalized	1	v/c	0.943	0.931	0.379	0.948	0.951			0.020	0.027
Signalized Control C		Signalized	1	v/c	0.943	0.931	0.379	0.948	0.951	0.406	AM & PM v/c ≥ 0.9	0.020 00 (BG & TF), PM	0.027 Site Impact > 1.0%
11. Research Blvd / Site Driveway 12. Access Drive / Site Driveway 13. Research Blvd / Site Driveway 14. Research Blvd / Site Driveway 15. Research Blvd / Site Driv	10. Research Blvd / 1600 Research	Signalized Intersection AM PM SAT	(max) v/c	v/c LOS	0.943 E	0.931 E	0.379 A 1	0.948 E .0. Rese	0.951 E arch Blv	0.406 A /d / 160	AM & PM v/c ≥ 0.9 I O Research Drive	0.020 90 (BG & TF), PM Mitigation Require	0.027 Site Impact > 1.0%
Intersection Inte	10. Research Blvd / 1600 Research Driveway	Signalized Intersection AM PM SAT	v/c 0.89	v/c LOS	0.943 E 537	0.931 E 584	0.379 A 136	0.948 E 0. Rese 544	0.951 E arch Blv 609	0.406 A /d / 160 165	AM & PM v/c ≥ 0.9 F O Research Drive 7	0.020 90 (BG & TF), PM Mitigation Require eway 25	0.027 Site Impact > 1.0% ed
1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1500		Signalized Intersection AM PM SAT 1500 1500 1500	v/c 0.89	v/c LOS	0.943 E 537	0.931 E 584	0.379 A 136	0.948 E .0. Rese 544	0.951 E arch Blv 609	0.406 A /d / 160 165	AM & PM v/c ≥ 0.9 F O Research Drive 7 0.005	0.020 90 (BG & TF), PM Mitigation Require eway 25 0.017	0.027 Site Impact > 1.0% ed 29 0.019
Two-Way Stop Control Two-Way Side Driveway Two-Way Side Driveway 1.00		Signalized Intersection AM PM SAT 1500 1500 1500 Signalized	v/c 0.89	V/c LOS	0.943 E 537 0.358	0.931 E 584 0.389	0.379 A 136 0.091	0.948 E 0. Rese 544 0.363	0.951 E arch Blv 609 0.406	0.406 A /d / 160 165 0.110	AM & PM v/c ≥ 0.5 0 Research Drive 7 0.005	0.020 00 (BG & TF), PM Mitigation Require eway 25 0.017 ≤ 0.89, Site Impact < 1	0.027 Site Impact > 1.0% ed 29 0.019
1.00 1.00		Signalized Intersection AM PM SAT 1500 1500 1500 Signalized Intersection	v/c 0.89 (max)	V/c LOS	0.943 E 537 0.358	0.931 E 584 0.389	0.379 A 136 0.091	0.948 E 0. Rese 544 0.363 A	0.951 E arch Blv 609 0.406 A	0.406 A rd / 160 165 0.110 A	AM & PM v/c ≥ 0.9 0 Research Drive 7 0.005 v/c No Interes	0.020 00 (BG & TF), PM Mitigation Require eway 25 0.017 ≤ 0.89, Site Impact < 1	0.027 Site Impact > 1.0% ed 29 0.019
1.2. Access Drive / Site Driveway AM PM SAT 1600 160	Driveway	Signalized Intersection AM PM SAT 1500 1500 1500 Signalized Intersection AM PM SAT	v/c 0.89 (max)	V/c LOS CLV V/c LOS	0.943 E 537 0.358 A	0.931 E 584 0.389 A	0.379 A 136 0.091 A	0.948 E 0. Rese 544 0.363 A 11.	0.951 E arch Blv 609 0.406 A Resear	0.406 A 7d / 160 165 0.110 A ch Blvd	AM & PM v/c ≥ 0.5 0 Research Drive 7 0.005 v/c: No Inters / Site Driveway 25	0.020 00 (BG & TF), PM Witigation Require eway 25 0.017 0.089, Site Impact < 1 section Mitigation	0.027 Site Impact > 1.0% ed 29 0.019 0.0% Required
Access Drive / Site Driveway Am PM SAT 1600 1	Driveway	Signalized Intersection	v/c 0.89 (max) v/c 0.89	CLV v/c LOS	0.943 E 537 0.358 A	0.931 E 584 0.389 A	0.379 A 136 0.091 A	0.948 E 0. Rese 544 0.363 A 11.	0.951 E arch Blv 609 0.406 A Resear	0.406 A 7d / 160 165 0.110 A ch Blvd	AM & PM v/c ≥ 0.5 0 Research Drive 7 0.005 v/c No Inters / Site Driveway 25 0.016	0.020 00 (BG & TF), PM Witigation Require eway 25 0.017 <0.09, Site Impact < 1 section Mitigation 105 0.065	0.027 Site Impact > 1.0% ed 29 0.019 0.0% Required 115 0.072
1600 1600	Driveway	Signalized Intersection AM PM SAT 1500 1500 1500 Signalized Intersection AM PM SAT 1600 1600 1600 Two-Way Stop	v/c 0.89 (max) v/c 0.89	v/c LOS CLV v/c LOS CLV v/c	0.943 E 537 0.358 A 594 0.371	584 0.389 A 582 0.364	0.379 A 1 136 0.091 A 162 0.101	0.948 E 0. Rese 544 0.363 A 11. 619 0.387	0.951 E arch Blv 609 0.406 A Resear 687 0.429	0.406 A 7d / 160 165 0.110 A ch Blvd 277 0.173	AM & PM v/c ≥ 0.5 O Research Drive 7 0.005 No Inters / Site Driveway 25 0.016	0.020 00 (BG & TF), PM Witigation Require eway 25 0.017 <0.089, Site Impact < 1 section Mitigation 105 0.065 <0.09, Site Impact < 1	0.027 Site Impact > 1.0% dd 29 0.019 0.0% Required 115 0.072 0.0%
AM PM SAT SAT Site Impact > 10.0% Forward SAT Site Im	Driveway 11. Research Blvd / Site Driveway	Signalized Intersection AM PM SAT 1500 1500 1500 Signalized Intersection AM PM SAT 1600 1600 1600 Two-Way Stop Control	v/c 0.89 (max) v/c 0.89 (max)	v/c LOS CLV v/c LOS CLV v/c	0.943 E 537 0.358 A 594 0.371	584 0.389 A 582 0.364	0.379 A 1 136 0.091 A 162 0.101	0.948 E 0. Rese 544 0.363 A 11. 619 0.387 A	0.951 E arch Blv 609 0.406 A Resear 687 0.429 A	0.406 A rd / 160 165 0.110 A ch Blvd 277 0.173 A	AM & PM v/c ≥ 0.5 O Research Drive 7 0.005 No Inters / Site Driveway 25 0.016 Wc Mitigation	0.020 00 (BG & TF), PM Witigation Require eway 25 0.017 <0.089, Site Impact < 1 section Mitigation 105 0.065 <0.09, Site Impact < 1	0.027 Site Impact > 1.0% dd 29 0.019 0.0% Required 115 0.072 0.0%
Control Los A A A A A A A A A	Driveway	Signalized Intersection AM PM SAT 1500 1500 1500 Signalized Intersection AM PM SAT 1600 1600 1600 Two-Way Stop Control AM PM SAT	v/c 0.89 (max) v/c 0.89 (max)	v/c LOS CLV v/c LOS CLV v/c LOS	0.943 E 537 0.358 A 594 0.371	0.931 E 584 0.389 A 582 0.364	0.379 A 136 0.091 A 162 0.101 A	0.948 E 0. Rese 544 0.363 A 11. 619 0.387 A	0.951 E arch Blv 609 0.406 A Resear 687 0.429 A . Access	0.406 A od / 160 165 0.110 A ch Blvd 277 0.173 A s Drive /	AM & PM v/c ≥ 0.6 O Research Drive 7 0.005 No Inters / Site Driveway 25 0.016 Mitigation / Site Driveway	0.020 0 (BG & TF), PM Wittigation Require eway 25 0.017 <0.89, Site Impact <1 105 0.065 <0.89, Site Impact <1 on Provided, but No	0.027 Site Impact > 1.0% 0.07 0.019 0.0% Required 115 0.072 0.0% t Required
AM PM SAT 1500	Driveway 11. Research Blvd / Site Driveway	Signalized Intersection	v/c 0.89 (max) v/c 0.89 (max) v/c 0.79	CLV v/c LOS	0.943 E 537 0.358 A 594 0.371 A 203	0.931 E 584 0.389 A 582 0.364 A 189	0.379 A 136 0.091 A 162 0.101 A	0.948 E 0. Rese 544 0.363 A 11. 619 0.387 A	0.951 E arch Blv 609 0.406 A Resear 687 0.429 A . Access	0.406 A rd / 160 165 0.110 A ch Blvd 277 0.173 A s Drive / 802	AM & PM v/c ≥ 0.6 O Research Drive 7 0.005 No Inters / Site Driveway 25 0.016 Mitigation / Site Driveway 40 0.025	0.020 00 (BG & TF), PM Wiltigation Require eway 25 0.017 ≤ 0.89, Site Impact < 1 section Mitigation 105 0.065 ≤ 0.89, Site Impact < 1 n Provided, but No	0.027 Site impact > 1.0% dd 29 0.019 0.0% Required 115 0.072 0.0% tr Required 729 0.455
1500 1000 1000	Driveway 11. Research Blvd / Site Driveway	Signalized Intersection	v/c 0.89 (max) v/c 0.89 (max) v/c 0.79	CLV V/C LOS	0.943 E 537 0.358 A 594 0.371 A 203 0.127	584 0.389 A 582 0.364 A	136 0.091 A 162 0.101 A	0.948 E 0. Rese 544 0.363 A 11. 619 0.387 A 12 243 0.152	0.951 E arch Blv 609 0.406 A Resear 687 0.429 A Access 712	0.406 A d / 160 165 0.110 A ch Blvd 277 0.173 A s Drive / 802 0.501	AM & PM v/c ≥ 0.6 O Research Drive 7 0.005 No Inters / Site Driveway 25 0.016 Mitigation / Site Driveway 40 0.025 v/c ≤ 0.79, F	0.020 00 (BG & TF), PM Wittigation Require eway 25 0.017 ≤ 0.89, Site Impact < 1 section Mitigation 105 0.065 ≤ 0.89, Site Impact < 1 Provided, but No 523 0.327	0.027 Site impact > 1.0% dd 29 0.019 0.0% Required 115 0.072 0.0% at Required 729 0.455 pact > 10.0%
Signalized Intersection Signalized Inte	11. Research Blvd / Site Driveway 12. Access Drive / Site Driveway	Signalized Intersection	v/c 0.89 (max) v/c 0.89 (max) v/c 0.79 (max)	CLV V/C LOS	0.943 E 537 0.358 A 594 0.371 A 203 0.127	584 0.389 A 582 0.364 A	136 0.091 A 162 0.101 A	0.948 E 0. Rese 544 0.363 A 11. 619 0.387 A 12 243 0.152 A	0.951 E arch Blv 609 0.406 A Resear 687 0.429 A . Access 712 0.445 A	0.406 A d / 160 165 0.110 A ch Blvd 277 0.173 A s Drive / 802 0.501 A	AM & PM v/c ≥ 0.6 O Research Drivi 7 0.005 No Inters / Site Driveway 25 0.016 Mitigation / Site Driveway 40 0.025 v/c ≤ 0.79, F Private Road - Mi	0.020 00 (BG & TF), PM Wiftigation Requires eway 25 0.017 ≤ 0.89, Site Impact < 1 5 cection Mitigation 105 0.065 ≤ 0.89, Site Impact < 1 10 Provided, but No 523 0.327 PM & SAT Site Imp	0.027 Site impact > 1.0% dd 29 0.019 0.0% Required 115 0.072 0.0% at Required 729 0.455 pact > 10.0%
LA. West Gude Dr / Piccard Dr AM PM SAT 1015 389 872 1023 398 1 8 9	11. Research Blvd / Site Driveway 12. Access Drive / Site Driveway 13. Shady Grove Rd / Darnestown	Signalized Intersection	v/c 0.89 (max) v/c 0.89 (max) v/c 0.79 (max)	CLV v/c LOS CLV v/c LOS CLV v/c LOS	537 0.358 A 594 0.371 A 203 0.127 A	584 0.389 A 582 0.364 A 189 0.118	0.379 A 136 0.091 A 162 0.101 A 73 0.046 A	0.948 E 0. Resee 544 0.363 A 11. 619 0.387 A 12 243 0.152 A 13. S	0.951 E arch Blv 609 0.406 A Resear 687 0.429 A . Access 712 0.445 A hady Gr	0.406 A rd / 160 165 0.110 A ch Blvd 277 0.173 A s Drive / 802 0.501 A rove Rd	AM & PM v/c ≥ 0.5 O Research Drive 7 0.005 No Inters / Site Driveway 25 0.016 Witigation / Site Driveway 40 0.025 v/c ≤ 0.79, F Private Road - Mi / Darnestown R	0.020 00 (BG & TF), PM Wiltigation Requires eway 25 0.017 ≤ 0.89, Site Impact < 1 section Mitigation 105 0.065 ≤ 0.89, Site Impact < 1 n Provided, but No 523 0.327 PM & SAT Site Im tiggation Provided, dd	0.027 Site impact > 1.0% dd 29 0.019 0.0% Required 115 0.072 0.0% tr Required 729 0.455 0.act > 10.0% but Not Required
AM	11. Research Blvd / Site Driveway 12. Access Drive / Site Driveway	Signalized Intersection	v/c 0.89 (max) v/c 0.89 (max) v/c 0.89 (max) v/c 0.79 (max) v/c 0.89	CLV v/c LOS CLV v/c LOS CLV v/c LOS CLV v/c LOS	537 0.358 A 594 0.371 A 203 0.127 A	0.931 E 584 0.389 A 582 0.364 A 189 0.118 A	0.379 A 11 136 0.091 A 162 0.101 A 73 0.046 A	0.948 E 0. Rese 544 0.363 A 11. 619 0.387 A 12 243 0.152 A 13. S	0.951 E arch Blv 609 0.406 A Resear 687 0.429 A . Access 712 0.445 A hady Gr 1181	0.406 A rd / 160 165 0.110 A ch Blvd 277 0.173 A s Drive 802 0.501 A rove Rd 911	AM & PM v/c ≥ 0.6 O Research Drive 7 0.005 No Inters / Site Driveway 25 0.016 Mitigatior / Site Driveway 40 0.025 v/c ≤ 0.79, F	0.020 10 (BG & TF), PM Wittigation Requires Eway 25 0.017 So.89, Site Impact < 1 105 0.065 0.065 0.089, Site Impact < 1 1 Provided, but No 523 0.327 M & SAT Site Impitigation Provided, dd 10 10	0.027 Site Impact > 1.0% 29 0.019 0.0% Required 115 0.072 0.0% t Required 729 0.455 pact > 10.0% but Not Required
1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1650	11. Research Blvd / Site Driveway 12. Access Drive / Site Driveway 13. Shady Grove Rd / Darnestown	Signalized Intersection	v/c 0.89 (max) v/c 0.89 (max) v/c 0.89 (max) v/c 0.79 (max) v/c 0.89	CLV V/C LOS	537 0.358 A 594 0.371 A 203 0.127 A	584 0.389 A 582 0.364 A 189 0.118 A	0.379 A 136 0.091 A 162 0.101 A 73 0.046 A 894 0.596	0.948 E 0. Rese 544 0.363 A 11. 619 0.387 A 12 243 0.152 A 13. § 1370 0.913	0.951 E arch Blv 609 0.406 A Resear 687 0.429 A Access 712 0.445 A hady Gr 1181 0.787	0.406 A rd / 160 165 0.110 A ch Blvd 277 0.173 A 802 0.501 A rove Rd 911 0.607	AM & PM v/c ≥ 0.5 O Research Drive 7 0.005 No Inters / Site Driveway 25 0.016 Mitigatior / Site Driveway 40 0.025 v/c ≤ 0.79, Private Road - Mi / Darnestown R 4 0.002 AM v/c ≥ 0.90	0.020 0 (BG & TF), PM Wittigation Require eway 25 0.017 <0.089, Site Impact <1 105 0.065 <0.089, Site Impact <1 10 Provided, but No 523 0.327 -M & SAT Site Impitigation Provided, d 10 0.006 (BG&TF), AM Site	0.027 Site Impact > 1.0% dd 29 0.019 0.0% Required 115 0.072 0.0% t Required 729 0.455 Dact > 10.0% but Not Required 17 0.011 Impact < 1.0%
Signalized Intersection Signalized Intersection Signalized Intersection Intersect	11. Research Blvd / Site Driveway 12. Access Drive / Site Driveway 13. Shady Grove Rd / Darnestown Rd	Signalized Intersection	v/c 0.89 (max) v/c 0.89 (max) v/c 0.79 (max) v/c 0.89 (max)	CLV V/C LOS	537 0.358 A 594 0.371 A 203 0.127 A	584 0.389 A 582 0.364 A 189 0.118 A	0.379 A 136 0.091 A 162 0.101 A 73 0.046 A 894 0.596	0.948 E 0. Rese 544 0.363 A 11. 619 0.387 A 12 243 0.152 A 13.5 1370 0.913 E	0.951 E arch Blv 609 0.406 A Resear 687 0.429 A . Access 712 0.445 A hady Gr 1181 0.787	0.406 A rd / 160 165 0.110 A ch Blvd 277 0.173 A s Drive / 802 0.501 A rove Rd 911 0.607 B	AM & PM v/c ≥ 0.8 O Research Drive 7 0.005 No Inters / Site Driveway 25 0.016 Mitigation / Site Driveway 40 0.025 V/c ≥ 0.79, F Private Road - Mi 0.002 AM v/c ≥ 0.90 No Inters	0.020 0 (BG & TF), PM Wittigation Require eway 25 0.017 <0.089, Site Impact <1 105 0.065 <0.089, Site Impact <1 10 Provided, but No 523 0.327 -M & SAT Site Impitigation Provided, d 10 0.006 (BG&TF), AM Site	0.027 Site Impact > 1.0% dd 29 0.019 0.0% Required 115 0.072 0.0% t Required 729 0.455 Dact > 10.0% but Not Required 17 0.011 Impact < 1.0%
LoS A B A A B A B A B A B A B A B B	11. Research Blvd / Site Driveway 12. Access Drive / Site Driveway 13. Shady Grove Rd / Darnestown	Signalized Intersection	v/c 0.89 (max) v/c 0.89 (max) v/c 0.79 (max) v/c 0.79 (max)	CLV V/C LOS	0.943 E 537 0.358 A 594 0.371 A 203 0.127 A 1366 0.911 E	584 0.389 A 582 0.364 A 189 0.118 A	136 0.091 A 162 0.101 A 73 0.046 A	0.948 E 0. Rese 544 0.363 A 11. 619 0.387 A 12 243 0.152 A 13. S 1370 0.913 E	0.951 E 609 0.406 A Resear 687 0.429 A . Access 712 0.445 A hady Gr 1181 0.787 C	0.406 A rd / 160 165 0.110 A ch Blvd 277 0.173 A s Drive / 802 0.501 A rove Rd 911 0.607 B Gude D	AM & PM v/c ≥ 0.6 O Research Drive 7 0.005 V/c No Inters / Site Driveway 25 0.016 Witigation Site Driveway 40 0.025 v/c ≤ 0.79, F Private Road - Mi / Darnestown R 4 0.002 AM v/c ≥ 0.90 AM v/c ≥ 0.90 or / Piccard Dr	0.020 00 (BG & TF), PM Wiltigation Require eway 25 0.017 ≤ 0.89, Site Impact < 1 0.065 ≤ 0.89, Site Impact < 1 10 Provided, but No 523 0.327 PM & SAT Site Impattigation Provided, d 10 0.006 (BG&TF), AM Site section Mitigation	0.027
AM PM SAT 15. West Gude Dr / Gaither Rd 1500 1	11. Research Blvd / Site Driveway 12. Access Drive / Site Driveway 13. Shady Grove Rd / Darnestown Rd	Signalized Intersection	v/c 0.89 (max) v/c 0.89 (max) v/c 0.79 (max) v/c 0.79 (max) v/c 0.89 (max)	CLV V/C LOS CLV V/C LOS CLV CLV CCLV CCLV CCLV CCLV CCLV CCLV	537 0.358 A 594 0.371 A 203 0.127 A 1366 0.911 E	584 0.389 A 582 0.364 A 189 0.118 A 1171 0.781 C	0.379 A 136 0.091 A 162 0.101 A 73 0.046 A 894 0.596 A	0.948 E 0. Rese 544 0.363 A 11. 619 0.387 A 122 243 0.152 A 13. S 1370 0.913 E	0.951 E arch Blv 609 0.406 A Resear 687 0.429 A . Access 712 0.445 A hady Gr 1181 0.787 C 4. West	0.406 A rd / 160 165 0.110 A ch Blvd 277 0.173 A s Drive / 802 0.501 A rove Rd 911 0.607 B Gude E 398	AM & PM v/c ≥ 0.6 O Research Drive 7 0.005 V/c No Inters / Site Driveway 25 0.016 Witigation Site Driveway 40 0.025 v/c ≤ 0.79, F Private Road - Mi / Darnestown R 4 0.002 AM v/c ≥ 0.90 AM v/c ≥ 0.90 or / Piccard Dr	0.020 00 (BG & TF), PM Wittigation Require eway 25 0.017 <0.89, Site Impact <1 section Mitigation 105 0.065 <0.89, Site Impact <1 n Provided, but No 523 0.327 -M & SAT Site Impitigation Provided, d 10 0.006 (BG&TF), AM Site section Mitigation	0.027 Site Impact > 1.0% detection 29 0.019 0.0% Required 115 0.072 0.0% tr Required 729 0.455 pact > 10.0% but Not Required 17 0.011 Impact < 1.0% Required
1500 1500	11. Research Blvd / Site Driveway 12. Access Drive / Site Driveway 13. Shady Grove Rd / Darnestown Rd	Signalized Intersection	v/c 0.89 (max) v/c 0.89 (max) v/c 0.79 (max) v/c 0.79 (max) v/c 0.89 (max)	CLV V/C LOS	0.943 E 537 0.358 A 594 0.371 A 203 0.127 A 1366 0.911 E 871 0.581	584 0.389 A 582 0.364 A 189 0.118 A 1171 0.781 C	0.379 A 136 0.091 A 162 0.101 A 73 0.046 A 894 0.596 A	0.948 E 0. Rese 544 0.363 A 11. 619 0.387 A 122 243 0.152 A 13. S 1370 0.913 E 1. 872 0.581	0.951 E arch Blv 609 0.406 A Resear 687 0.429 A .Access 712 0.445 A hady Gr 1181 0.787 C 4. West 1023 0.682	0.406 A rd / 160 165 0.110 A ch Blvd 277 0.173 A s Drive / 802 0.501 A rove Rd 911 0.607 B Gude E 398 0.265	AM & PM v/c ≥ 0.6 O Research Drive 7 0.005 No Inters / Site Driveway 25 0.016 Witigation / Site Driveway 40 0.025 v/c ≤ 0.79, Frivate Road - Mil / Darnestown R 4 0.002 AM v/c ≥ 0.90 No Inters or / Piccard Dr 1 -	0.020 0 (BG & TF), PM Wittigation Require Eway 25 0.017 6 0.89, Site Impact < 1 105 0.065 6 0.89, Site Impact < 1 10 Provided, but No 523 0.327 PM & SAT Site Impattigation Provided, d 10 0.006 (BG&TF), AM Site section Mitigation	0.027 Site Impact > 1.0% 29 0.019 0.0% Required 115 0.072 0.0% 729 0.455 0.455 0.011 17 0.011 Impact < 1.0% Required
Signalized Intersection Signalized Intersection Signalized Intersection Intersecti	11. Research Blvd / Site Driveway 12. Access Drive / Site Driveway 13. Shady Grove Rd / Darnestown Rd 14. West Gude Dr / Piccard Dr	Signalized Intersection	V/C 0.89 (max)	CLV V/C LOS	0.943 E 537 0.358 A 594 0.371 A 203 0.127 A 1366 0.911 E 871 0.581	584 0.389 A 582 0.364 A 189 0.118 A 1171 0.781 C	0.379 A 136 0.091 A 162 0.101 A 73 0.046 A 894 0.596 A	0.948 E 0. Rese 544 0.363 A 11. 619 0.387 A 12 243 0.152 A 13.5 1370 0.913 E 1 872 0.581 A	0.951 E arch Blv 609 0.406 A Resear 687 0.429 A . Access 712 0.445 A hady Gr 1181 0.787 C 4. West 1023 0.682 B	0.406 A rod / 160 165 0.110 A ch Blvd 277 0.173 A s Drive / 802 0.501 A rove Rd 911 0.607 B Gude C 398 0.265 A	AM & PM v/c ≥ 0.6 O Research Drive 7 0.005 No Inters 25 0.016 Mitigatior (Site Driveway 40 0.025 v/c ≥ 0.79, Frivate Road - Mi / Darnestown R 4 0.002 AM v/c ≥ 0.90 No Inters Or / Piccard Dr 1 - v/c No Inters	0.020 0 (BG & TF), PM Wittigation Require Eway 25 0.017 6 0.89, Site Impact < 1 105 0.065 6 0.89, Site Impact < 1 10 Provided, but No 523 0.327 PM & SAT Site Impattigation Provided, d 10 0.006 (BG&TF), AM Site section Mitigation	0.027 Site Impact > 1.0% 29 0.019 0.0% Required 115 0.072 0.0% 729 0.455 0.455 0.011 17 0.011 Impact < 1.0% Required
1.5 A B A A B B A A B B A A B B	11. Research Blvd / Site Driveway 12. Access Drive / Site Driveway 13. Shady Grove Rd / Darnestown Rd	Signalized Intersection	v/c 0.89 (max) v/c 0.89 (max) v/c 0.79 (max) v/c 0.79 (max) v/c 0.89 (max)	V/C LOS CLV V/C CLV V/C LOS CLV V/C CLV V/C	0.943 E 537 0.358 A 594 0.371 A 203 0.127 A 1366 0.911 E 871 0.581 A	0.931 E 584 0.389 A 582 0.364 A 189 0.118 A 1171 0.781 C	0.379 A 1136 0.091 A 162 0.101 A 73 0.046 A 894 0.596 A	0.948 E 0. Rese 544 0.363 A 11. 619 0.387 A 12 243 0.152 A 13.5 1370 0.913 E 1. 872 0.581 A 1.	E arch Blv 609 A Resear 687 712 0.445 A A Ccess 712 0.445 A 1023 C C C C C C C C C C C C C C C C C C C	0.406 A d / 160 165 0.110 A ch Blvd 277 0.173 A s Drive / 802 0.501 A rove Rd 911 0.607 B Gude D 398 0.265 A Gude D	AM & PM v/c ≥ 0.8 O Research Drivi 7 0.005 No Inters / Site Driveway 25 0.016 Mitigation / Site Driveway 40 0.025 V/c ≤ 0.79, F Private Road - Mi / Darnestown R 4 0.002 AM v/c ≥ 0.90 No Inters or / Piccard Dr V/c No Inters or / Gaither Rd	0.020 0 (BG & TF), PM Wittigation Require eway 25 0.017 0.089, Site Impact <1 section Mitigation 105 0.065 0.065 0.065 0.089, Site Impact <1 1 n Provided, but No 523 0.327 0.327 0.006 0.	0.027
16. W. Montgomery Ave (MD 28) / AM PM SAT 1650 1	11. Research Blvd / Site Driveway 12. Access Drive / Site Driveway 13. Shady Grove Rd / Darnestown Rd 14. West Gude Dr / Piccard Dr	Signalized Intersection	v/c 0.89 (max)	CLV V/C LOS CLV V/C LOS CLV V/C LOS CLV CLV CLOS CLV CLV CCLV CCLV CCLV CCLV CCLV CCLV	0.943 E 537 0.358 A 594 0.371 A 203 0.127 A 1366 0.911 E 871 0.581 A	0.931 E 584 0.389 A 582 0.364 A 189 0.118 A 1171 0.781 C	0.379 A 1 136 0.091 A 162 0.101 A 73 0.046 A 894 0.596 A 389 0.259 A	0.948 E 0. Rese 544 0.363 A 11. 619 0.387 A 12243 0.152 A 13. S 1370 0.913 E 1. 872 0.581 A	0.951 E 609 0.406 A Resear 687 A . Access 712 0.445 A hady Gr 1181 0.787 C 4. Wests 1023 0.682 B 5. West	0.406 A rd / 160 165 0.110 A ch Blvd 277 0.173 A s Drive / 802 0.501 A rove Rd 911 0.607 B Gude D 398 0.265 A Gude D 445	AM & PM v/c ≥ 0.6 O Research Drive 7 0.005 V/c No Inters / Site Driveway 25 0.016 Witigation / Site Driveway 40 0.025 v/c ≤ 0.79, F Private Road - Mit / Darnestown R 4 0.002 AM v/c ≥ 0.90 No Inters or / Piccard Dr 1 V/c No Inters r / Gaither Rd 1	C.020 0 (BG & TF), PM Wittigation Require Eway 25 0.017 0.08, Site Impact < 1 0.065 0.065 0.08, Site Impact < 1 0 Provided, but No 523 0.327 0.327 0.006 0.006 0.006 0.006 0.006 0.006 0.006 0.005	0.027 Site Impact > 1.0% 29 0.019 0.0% 115 0.072 0.072 0.0% 729 0.455 0.455 0.011 17 0.011 Impact < 1.0% Required 9 0.006 0.0% Required
Laird St	11. Research Blvd / Site Driveway 12. Access Drive / Site Driveway 13. Shady Grove Rd / Darnestown Rd 14. West Gude Dr / Piccard Dr	Signalized Intersection	v/c 0.89 (max)	CLV V/C LOS	0.943 E 537 0.358 A 594 0.371 A 203 0.127 A 1366 0.911 E 871 0.581 A	0.931 E 584 0.389 A 582 0.364 A 189 0.118 A 1171 0.781 C	0.379 A 1 136 0.091 A 162 0.101 A 894 0.596 A 389 0.259 A 435 0.29	0.948 E 0. Rese 544 0.363 A 11. 619 0.387 A 12243 0.152 A 13. S 1370 0.913 E 1. 872 0.581 A	E 609 0.406 A Resear 687 A A A A A Ceest 1181 0.787 C C 4.4 West 1 1023 0.682 B B B S West 1 1000 0.667	0.406 A rod / 160 165 0.110 A ch Blvd 277 0.173 A S Drive / 802 0.501 A rove Rd 911 0.607 B Gude D 398 0.265 A Gude D 445 0.297	AM & PM v/c ≥ 0.6 O Research Drive 7 0.005 V/c No Inters / Site Driveway 25 0.016 Mitigation Site Driveway 40 0.025 V/c ≤ 0.79, F Private Road - Mi / Darnestown R 4 0.002 AM v/c ≥ 0.90 Mo Inters O	0.020 00 (BG & TF), PM Wittigation Require eway 25 0.017 ≤ 0.89, Site Impact < 1 0.065 ≤ 0.89, Site Impact < 1 0.015 0.065 ≤ 0.89, Site Impact < 1 0.006 0.327 PM & SAT Site Impact < 1 0.006 (BG&TF), AM Site section Mitigation 8 0.005 ≤ 0.89, Site Impact < 1 0.006 S 0.89, Site Impact < 1 0.006	0.027
Signalized Intersection Signalized Intersection Intersec	11. Research Blvd / Site Driveway 12. Access Drive / Site Driveway 13. Shady Grove Rd / Darnestown Rd 14. West Gude Dr / Piccard Dr 15. West Gude Dr / Gaither Rd	Signalized Intersection	V/C 0.89 (max) V/C 0.89 (max) V/C 0.89 (max) V/C 0.89 (max)	CLV V/C LOS	0.943 E 537 0.358 A 594 0.371 A 203 0.127 A 1366 0.911 E 871 0.581 A	0.931 E 584 0.389 A 582 0.364 A 189 0.118 A 1171 0.781 C	0.379 A 1 136 0.091 A 162 0.101 A 73 0.046 A 894 0.596 A 389 0.259 A	0.948 E 0. Rese 544 0.363 A 11. 619 0.387 A 12 243 0.152 A 13.S 1370 0.913 E 1. 872 0.581 A	E 609 0.406 A Resear 687 A . Access 1181 0.787 C . A hady G 1181 0.787 C . B B B S . West 1000 0.667 B	0.406 A rod / 160 165 0.110 A ch Blvd 277 0.173 A s Drive / 802 0.501 A rove Rd 911 0.607 B Gude D 398 0.265 A Gude D 445 0.297 A	AM & PM v/c ≥ 0.6 O Research Drive 7 0.005 No Inters / Site Driveway 25 0.016 Mitigation / Site Driveway 40 0.025 v/c ≤ 0.79, F Private Road - Mi / Darnestown R 4 0.002 AM v/c ≥ 0.90 No Inters or / Piccard Dr 1	0.020 0 (BG & TF), PM Wittigation Require eway 25 0.017 ≤ 0.89, Site Impact < 1 105 0.065 ≤ 0.89, Site Impact < 1 10 Provided, but No 223 0.327 -M & SAT Site Implitigation Provided, d 10 0.006 (BG&TF), AM Site section Mitigation	0.027
Signalized LoS B D A B D A B D A No Intersection Mitigation Required	11. Research Blvd / Site Driveway 12. Access Drive / Site Driveway 13. Shady Grove Rd / Darnestown Rd 14. West Gude Dr / Piccard Dr 15. West Gude Dr / Gaither Rd 16. W. Montgomery Ave (MD 28) /	Signalized Intersection	\(\sigma\)	CLV	0.943 E 537 0.358 A 594 0.371 A 203 0.127 A 1366 0.911 E 871 0.581 A	0.931 E 584 0.389 A 582 0.364 A 189 0.118 A 1171 0.781 C 1015 0.677 B	0.379 A 1 136 0.091 A 162 0.101 A 73 0.046 A 894 0.596 A 389 0.259 A	0.948 E 0. Rese 544 0.363 A 11. 619 0.387 A 12 243 0.152 A 13. S 1370 0.913 E 1. 872 0.581 A 1.6 865 0.577 A	Earch Bly 687 0.429 A Access 712 0.445 A hady Gri 1181 0.787 C 4. West 1002 B 5. West B Montgoin	0.406 A rd / 160 165 0.110 A ch Blvd 277 0.173 A s Drive / 802 0.501 A rove Rd 911 0.607 B Gude D 398 0.265 A Gude D 445 0.297 A	AM & PM v/c ≥ 0.6 O Research Drive 7 0.005 No Inters / Site Driveway 25 0.016 Mitigation / Site Driveway 40 0.025 v/c ≤ 0.79, Frivate Road - Mi / Darnestown R 4 0.002 AM v/c ≥ 0.90 No Inters or / Piccard Dr 1 - v/c No Inters r / Gaither Rd 1 0.001 V/c No Inters r / Gaither Rd 1 0.001	0.020 00 (BG & TF), PM Wittigation Require eway 25 0.017 0.089, Site Impact < 1 0.065 0.065 0.089, Site Impact < 1 0.07 0.084 0.006	0.027
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1 W. Montgomery Ave (MD 28)/I-270 NB Ramp/Nelson St v/c Threshold: 0.99 Peak: AM PM SAT **CLV:** 927 | 1376 963

v/c: 0.618 0.917 0.688

2 West Montgomery Ave (MD 28)/I-270 SB Ramps v/c Threshold: 0.99 Peak: AM PM SAT **CLV:** 1166 | 1266 766 v/c: 0.707 0.767 0.479

v/c Threshold: 0.89 Peak: AM PM SAT CLV: 1107 | 1507 | 1009 v/c: 0.692 0.942 0.673

3 West Montgomery Ave (MD 28)/Hurley Ave

4 West Montgomery Ave (MD 28)/Research Blvd v/c Threshold: 0.89 Peak: AM PM SAT *CLV:* 875 1479 642 v/c: 0.583 0.986 0.459

5 West Montgomery Ave (MD 28)/Darnestown Rd v/c Threshold: 0.89 Peak: AM PM SAT **CLV:** 789 953 600 v/c: 0.493 0.596 0.400

6 West Montgomery Ave (MD 28)/W. Gude Dr v/c Threshold: 0.99 Peak: AM PM SAT **CLV:** 967 904 434 v/c: 0.586 0.548 0.263

7 West Montgomery Ave (MD 28)/Shady Grove Rd v/c Threshold: 0.99 Peak: AM PM SAT *CLV:* 1004 | 1225 | 577 v/c: 0.648 0.790 0.372 8 Research Blvd/Shady Grove Rd v/c Threshold: 0.89 Peak: AM PM SAT *CLV:* 1239 | 1307 | 550 v/c: 0.799 0.843 0.355 9 Research Blvd/West Gude Dr v/c Threshold: 0.89 Peak: AM PM SAT CLV: 1422 1427 568 **v/c:** 0.948 0.951 0.406 (10) Research Blvd/1600 Research Blvd Driveway v/c Threshold: 0.89 Peak: AM PM SAT **CLV**: 544 165 609 v/c: 0.363 0.406 0.110

13 Shady Grove Rd/Darnestown Rd v/c Threshold: 0.89 Peak: AM PM SAT **CLV:** 1370 | 1181 911 v/c: 0.913 0.787 0.607 14 West Gude Dr/Piccard Dr v/c Threshold: 0.89 PM | SAT Peak: AM **CLV:** 872 1023 | 398 v/c: 0.581 0.682 0.265 15 West Gude Dr/Gaither Rd v/c Threshold: 0.89 PM SAT Peak: AM **CLV:** 865 1000 445 v/c: 0.577 0.667 0.297 16 West Montgomery Ave (MD 28)/Laird St v/c Threshold: 0.89 Peak: AM PM SAT CLV: 1043 1484 815 v/c: 0.632 0.899 0.509 17 West Montgomery Ave (MD 28)/Great Falls Rd v/c Threshold: 0.99 Peak: AM PM SAT CLV: 1295 | 1440 | 1272 v/c: 0.809 0.900 0.848

Figure 8-5 2019 Total Future CLV & v/c



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The following intersection is not subject to mitigation requirements as it includes two private access roads. However, for informational purposes, it would operate at or within the congestion standards under total future conditions with the proposed redevelopment, and the CLV analyses indicate that the site impact at the intersections would result in a volume-to-capacity (v/c) increase of 10% or greater during one or more of the study peak hours:

• Intersection 12: Site Driveway / Hurley Avenue

- v/c Threshold: 0.79 (threshold exceeded if v/c reaches 0.80).
- 0.327 increase in v/c during the weekday PM peak hour.
- 0.455 increase in v/c during the mid-day Saturday peak hour.

The following intersections would operate beyond the acceptable volume-to-capacity thresholds under <u>both</u> background and total future conditions. Per the CTR, mitigation is required where the site impact results in a v/c increase of 0.01 (a full one percent) or greater between background and total future conditions.

Intersection 04: West Montgomery Avenue (MD 28) / Research Boulevard

- v/c Threshold: 0.89 (threshold exceeded if v/c reaches 0.90).
- During the PM peak hour, 0.943 v/c under background future conditions.
- During the PM peak hour, 0.986 v/c under total future conditions with the proposed redevelopment.
- Site impact of 0.043 (4.3%) increase in v/c during the weekday PM peak hour.
- Mitigation must be provided since site impact is 1.0 percent or greater.

Intersection 09: West Gude Drive / Research Boulevard

- v/c Threshold: 0.89 (threshold exceeded if v/c reaches 0.90).
- During the PM peak hour, 0.931 v/c under background future conditions.
- During the PM peak hour, 0.951 v/c under total future conditions with the proposed redevelopment.
- Site impact of 0.020 (2.0%) increase in v/c during the weekday PM peak hour.
- Mitigation must be provided since site impact is 1.0 percent or greater.

Intersection 13: Darnestown Road / Shady Grove Road

- v/c Threshold: 0.89 (threshold exceeded if v/c reaches 0.90).
- During the AM peak hour, 0.911 v/c under background future conditions.
- During the AM peak hour, 0.913 v/c under total future conditions with the proposed redevelopment.
- Site impact of 0.002 (0.2%) increase in v/c during the weekday AM peak hour.
- No mitigation required since the impact is less than 1.0 percent.

The following intersection would operate below the acceptable volume-to-capacity threshold under background conditions but beyond the acceptable threshold under total future conditions. Per the CTR, mitigation is required if the site impact results in a v/c increase of 0.10 (a full ten percent) or greater between background and total future conditions.

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- Intersection 03: W. Montgomery Ave. (MD 28)/Hurley Ave/Access Drive.
 - 0.89 (threshold exceeded if v/c reaches 0.90).
 - During the PM peak hour, 0.848 v/c under background future conditions.
 - During the PM peak hour, 0.942 v/c under total future conditions with the proposed redevelopment.
 - 0.094 (9.4%) increase in v/c during the weekday PM peak hour.
 - No mitigation required since the impact is less than 10.0 percent.

11.2 Proposed Mitigation

The Applicant proposes to provide the following mitigation in order to adequately mitigate the traffic related impacts that would result from the proposed redevelopment, and the resulting lane use and traffic controls, including these improvements, is depicted on Figure 11-1:

Intersection 04: West Montgomery Avenue (MD 28) / Research Boulevard

- The Applicant proposes to reconfigure the Research Boulevard approach at the intersection in order to convert the existing shared left-through lane into a dedicated left-turn lane. The existing right-turn lane is proposed to be converted into a shared left-through-right lane.
- As a result of the reconfiguration described above, the Research Boulevard approach would provide three (3) lanes to facilitate vehicles turning left onto West Montgomery Avenue (MD 28).
- Three (3) receiving lanes are currently present on West Montgomery Avenue (MD 28) to accommodate the proposed reconfiguration.
- In general, the proposed improvement would require a signal modification through cooperation with the Maryland State Highway Administration (SHA), new striping, possible wayfinding signage, and other accompanying improvements.
- The right-turn and through-movement traffic volumes account for only a minor portion (Approximately 5 percent) of the Research Boulevard approach traffic as shown below. Given the heavy left-turn volume when compared to the other movements, the priority should be given to the left-turn movement.

	Traffic Forecasts (Total Future 2019)									
		Res	earch Boule	evard Appro	oach					
Pe	eak Hour	Right	Through	Left	Total					
Weekday	Vehicles/Hour	16	0	327	343					
AM	% of Approach	4.7%	0.0%	95.3%	343					
Weekday	Vehicles/Hour	48	3	953	1004					
PM	% of Approach	4.8%	0.3%	94.9%	1004					
Saturday	Vehicles/Hour	18	3	244	265					
Mid-Day	% of Approach	6.8%	1.1%	92.1%	200					

Intersection 09: West Gude Drive / Research Boulevard

 The Applicant proposes to restripe the southbound Research Boulevard approach at the intersection in order to convert the currently striped out lane into a second left turn lane.

11.3 Additional Proposed Improvements

In addition to the required mitigation outlined in Section 11.2, the following additional improvements are proposed to be provided by the Applicant in order to further improve circulation, traffic operations, and both vehicular and pedestrian access in and around the site. The Applicant will work with the City and SHA to provide these improvements; however, it is noted that these improvements are not required to be provided in order to meet any of the mitigation requirements set forth in the City's CTR Guidelines. The resulting lane use and traffic controls, including these improvements, are depicted on Figure 11-1:

PROPOSED ADDITIONAL INTERSECTION IMPROVEMENTS

Intersection 03: W. Montgomery Ave. (MD 28)/Access Drive/Hurley Ave.

- The Access Drive is proposed to be restriped to the north of the intersection so that two (2) approach lanes will be available with adequate queue storage capacity. While there are separate right and left turn lanes on the Access Drive under existing conditions, the second lane only extends approximately 50 feet before tapering back to a single lane. The Access Drive approach would likely realize a significant improvement in operations with two egress lanes along the full length of the roadway and the lane use adjustment described below.
- At the intersection, the lane use is proposed to be reconfigured to provide one (1) shared through-right lane and one (1) left turn lane.
- In general, the adjusted lane use would require a signal modification through cooperation with the Maryland State Highway Administration (SHA), new striping, potential pedestrian crossing improvements, and other accompanying improvements.
- Since this approach is not the critical movement, the CLV and v/c would not change; however, traffic operations would be improved with the reduced queuing and more efficient lane use proposed.
- In response to SHA comments on the change in lane use proposed, a more in depth capacity analyses was performed using the Highway Capacity Manual methodology utilizing the Synchro (Version 9) traffic analysis software. A summary of the results from the HCM analysis are provided on Table 11-1 and detailed reports are provided in the Appendix. The results of these analyses indicate that the proposed improvements would provide significant

improvements for the side-street approaches of the Access Drive and Hurley Avenue. In addition to reduced average vehicle delays for the side street movements, the side street queues would be reduced as well. The proposed lane configuration of one (1) shared through-right lane and one (1) left turn lane on the Access Drive approach would result in overall better side street operations when compared to the existing lane use that includes a shared through-left land and a right turn lane.

Intersection 11: Site Driveway / Research Boulevard

- The Applicant is proposing to provide improvements along the site driveway on Research Boulevard that would result in separate left and right turn lanes at the intersection with Research Boulevard. This will reduce the potential for queuing on the driveway approach in the event that a left turning vehicle must wait for an available gap on Research Boulevard to exit the property.
- The Applicant also proposes that the left turn movement from the driveway onto Research Boulevard be restricted during the PM peak period due to the heavy traffic and queuing along Research Boulevard approaching MD 28 that occurs during the typical afternoon commuter peak period.

Intersection 12: Site Driveway / Access Drive

• The Access Drive is proposed to be restriped between the site driveway and West Montgomery Avenue (MD 28) to provide two (2) lanes serving egress traffic. While this will not have a direct impact on the CLV or the v/c ratio, this intersection is expected to operate at level-of-service "A" and is not subject to mitigation requirements.

Table 11-1 Critical Lane Volume Analyses Summary for Intersection Improvements

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Congestion Standard (Capacity)			Total Future				Development Impa	ct	Proposed Mitigation	Tota with N						
(Capacity)		AM PM SAT AM		AM	M PM SAT		Troposed Willigation	AM	PM	SAT						
(oupo	,			7		5711	7	1			omery Ave (N		ırlev Ave	7.001		5, (
AM PI	- 1	v/c	CLV	1072	1357	751	1107	1507	1009	35	150	258	Access Drive approach at MD 28 restriped to provide one (1) left turn lane and one (1) shared through-right lane. Leading left-turn phase	1105	1507	100
1600	1500	0.89	v/c	0.670	0.848	0.501	0.692	0.942	0.673	0.022	0.094	0.172		0.691	0.942	0.67
Signa		(max)						_		PM v/c ≥ 0	0.90 (TF), PM Site Im	pact = +9.4%	Drive section north of MD 28		_	
Interse	ection		LOS	В	D	А	В	E	В		Mitigation Require	d		В	Е	В
												НСМ Сар	acity Analyses (Synchro, Version 9)			
			٠.				•			-	Background Con	ditions	Total Future Conditions			
			Sti	ıdy I	ntei	rsect	tions	5				<u>PM</u>	# 014	PN	M	
								8	LOS (HCM) (s	Delay Queue)S Dela				
											Z (HCIVI) (S		= (11611) (3/4611) (10) = (116	M) (s/ve		(ft)
03. V	v. Mo	ntgom	ery A	ve (N	1D 28) / Hu	rley A	ve				78.3 #239	EB LT F 345.2 #377			313
	Prono	sed In	nrov	emer	nte					,		31.2 117 55.2	EBR C 29.4 117 900 EBR C EB EB E WBL T F 300.6 #465 WB T C WB T 215.4 WB T D 49.8 M144 NBL F NBL D 49.8 M144 NBL F			74
-	Provide	two (2) lanes	along t		length o	of Hurle	y Aven	ue betw	reen MD ction ovide Avenue	WOLT 5		WBL E			296
		he Site I									WB LT E	69.3 #205	WB T F 300.6 #465 F Q WB T C WB T C WB T C WB T C T C T C T C T C T C T C T C T C T	34.2	2 1	105
		the lane existing								rovide	WBR D	40.0 0 64.8	WBR D 40.0 86 TO WB F 215.4 WB E			
c	one (1) I	eft turn	lane a	nd one	(1) sha	red thro	ough-rig	ght lane	2.	<u> </u>	NBL D	51.7 m155	NBL D 49.8 m144 5 NBL F		-	#21
	Provide eaving		ing left	turn ph	nase to	clear ou	it the q	ueue or	n Hurley	Avenue	NBTR A	5.1 252	8 NBT A 7.6 321 E NBT E	59.4	4 #	97:
		are site.	ected le	eft turn	nhasee	enterina	the city	e off of	MD 28		100		NB B 10.5 NB E			
	rioviae	e a prote	ecteu ie	jt turri j	priuse e	entenny	the site	e 0jj 0j	IVID 26.		NB A SBL B	8.5 13.0 m4	NB B 10.5 NB E SBL F 329.9 m#168 SBL D			1#7
										ž		24.7 #791	SBT C 26.8 #767 8 SBT D			79!
							381 C 20.8 #707 6 381 B									
												24.6 19.5	SB D 38.7 SB D OVr D 46.0			
Note	s: (1)	Syncl	nro (\	/ersio	n 9) u	ised to	o calcı	ulate	нсм	Level of Serv	ice (LOS) and de			5-110		
											mery Ave (MD		<u> </u>			_
\neg									J V	7. WIOIILEO	ITICI Y AVE (IVIE	20// 11030	Work with the Maryland State Highway			
M PI	M SAT	v/c	CLV	861	1415	617	875	1479	642	14	64	25	Administration (SHA) to restripe the Research	816	1307	60
1	1												Boulevard Approach to Provide two (2) left-turn			
1500	1400	0.89	v/c	0.574	0.943	0.441	0.583	0.986	0.459	0.009	0.043	0.018	lanes and one (1) shared left-through-right lane.	0.544	0.871	0.4
Signa	lized	(max)								PM v/c ≥ 0.9	90 (BG&TF), PM Site	Impact > 1.0%	Upgrade traffic signal equipment as needed.			
Intersection LOS A E A A E A					Mitigation Require	d		A	D	1						
			,	•						09. F	Research Blvd ,	/ Gude Dr	•			
			CLV	1415	1397	530	1422	1427	568	7	30	38	Restripe the southbound Research Blvd	1313	1262	53
M PI	M SAT	v/c	CLV	1415	1397	550	1422	1427	306	/	30	30	approach to provide a 2nd left-turn lane onto	1515	1202	Э.
1500	1400	0.89	v/c	0 943	0 931	0.379	0 948	0.951	0.406	0.005	0.020	0.027	Gude Drive within the striped-out section of	0.875	0.841	0.3
15	17	0.03	., c	0.545	0.551	0.575	0.540	0.551	0.400	0.005	0.020	0.027	existing pavement. This improvement was previously planned by the 4 Research Place	0.075	7.041	0
Signa		(max)	LOS	Е	Е	А	Е	Е	А	AM & PM v/c ≥	0.90 (BG & TF), PM		development (approval recently expired).	D	D	١.
nterse	ection									44.5	Mitigation Require					
_			1							11. Res	earch Blvd / S	te Drivewa				
		,	CLV	594	582	162	619	687	277	25	105	115	Provide separate left turn lane on the site driveway, and restrict left-turn movement from	619	687	2
- 1	M SAT	v/c											the driveway onto Research Boulevard during			
1600	1600	0.89	v/c	0.371	0.364	0.101	0.387	0.429	0.173	0.016	0.065	0.072		0.387	0.429	0.1
Two-		(max)									v/c ≤ 0.89, Site Impact < 10	00%	crossing on driveway approach.			
			itigation Provided, but Not Required			А	Α	,								
										12. Ac	cess Drive / Sit	te Drivewa				
													Provide pedestrian sidewalk connection to the			
M PI	и SAT	v/c	CLV	203	189	73	243	712	802	40	523	729	adjacent Hotel property to the east. Sidewalk	243	712	8
1	- 1												to run along the north side of the "T"			
1600	1600	0.79	v/c	0.127	0.118	0.046	0.152	0.445	0.501	0.025	0.327	0.455	intersection. Second receiving faire added on	0.152	0.445	0.5
Two-	 Way	(max)								y/c ≤ 0.79	D, PM & SAT Site Imp	act > 10.0%	the Access Drive extending from the MD 28			
Stop C		ĺ .	LOS	Α	А	Α	Α	Α	А		Mitigation Provided,		intersection.	А	Α	Α

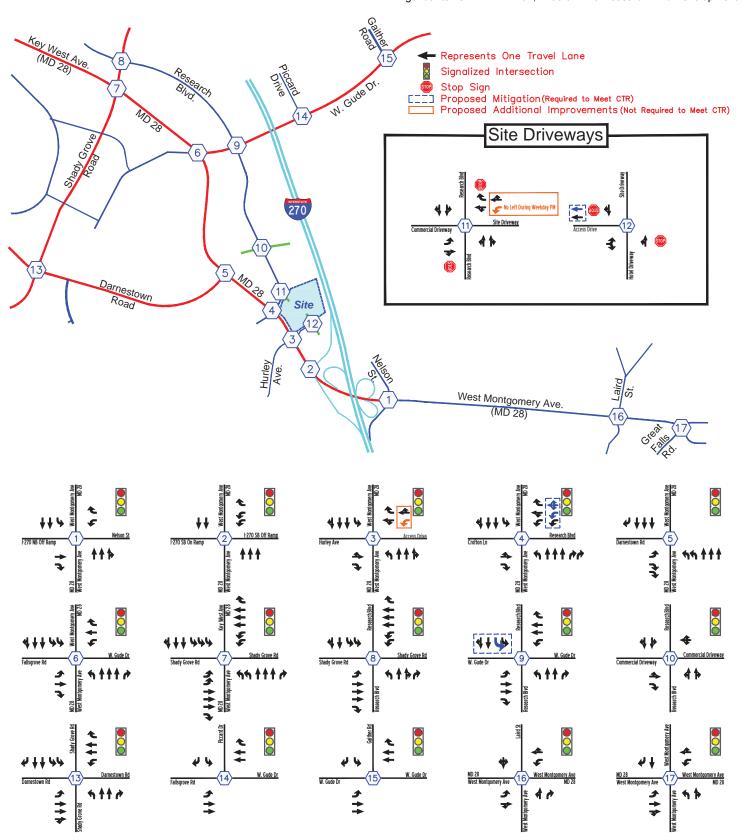


Figure 11-1
Total Future Lane Use and Traffic Controls with Mitigation & Additional Improvements
Research Row CTR

NORTH

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PROPOSED ADDITIONAL PEDESTRIAN IMPROVEMENTS

- A new interparcel sidewalk connection is proposed to be constructed that would connect the new and expansive pedestrian network within the Research Row development with the adjacent hotel property to the east. There are no current pedestrian interparcel connections in place between these properties.
- The Applicant is proposing to provide a pedestrian inter-parcel connection with the adjacent office development to the north of the site. No inter-parcel connections are provided under existing conditions. With the addition of retail uses on-site, the proposed inter-parcel connection will provide convenient access between the two properties for office users that will likely utilize many of the retail offerings throughout the day. This will likely result in some reduction in vehicular site traffic originating from the adjacent office development and/or other neighboring developments that would realize more convenient pedestrian access as a result of the improvement.
- The Applicant is proposing to relocate the existing (non ADA compliant) pedestrian ramp which is currently located along MD 28 near the Research Boulevard intersection. The ramp will be reconstructed to meet current ADA standards, and the ramp will be relocated further to the southeast to tie into the public sidewalk near the bus stop adjacent to the Access Drive. The reconstructed pedestrian ramp will tie into the improved internal pedestrian facilities within the site and will also provide a new and more direct route for pedestrians and bicyclists traveling to and from the north through the site.

COMPONENT 12 SUMMARY

Based on a review of the available traffic information and the future traffic forecasts, the following summarizes the results and conclusions of this Comprehensive Transportation Review (CTR):

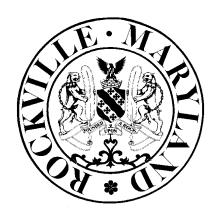
- 1. Sixteen of the seventeen study intersections currently operate at or within the acceptable thresholds during the weekday and Saturday peak hours studied. The Shady Grove Road / Darnestown Road intersection currently exceeds the acceptable v/c ratio of 0.89 during the AM peak hour; however, the intersection operates within the acceptable v/c ratio during the remaining peak hours studied.
- 2. The Shady Grove Road / Darnestown Road intersection would continue to exceed the acceptable v/c ratio of 0.89 during the AM peak hour, and the volume-to-capacity ratio at the MD 28 and West Gude Drive intersections along Research Boulevard would increase beyond the acceptable thresholds of 0.89 under background conditions during the AM and/or PM peak hours. All other study intersections would continue to operate at or within the acceptable thresholds during each peak hour studied under background conditions that include planned road improvements, additional traffic added to the road network by pipeline development, the regional growth in through traffic that would be realized along MD 28, and the potential site trips from the currently unutilized 105,000 SF of approved R&D uses.
- 3. The proposed redevelopment of the site, including construction of 102,535 SF of retail uses (before pass-by trip reduction) and the addition of 10,165 SF of new office space, would result in approximately 70 additional weekday AM, 646 additional weekday PM, and 865 additional mid-day Saturday peak hour trips being generated by the site under total future conditions in year 2019 when compared to the trip generation potential for the 105,000 SF of approved R&D uses that were recently demolished in April of 2015 in anticipation of the proposed redevelopment.
- 4. Consistent with both existing and background future conditions, the Shady Grove Road / Darnestown Road intersection would continue to exceed the acceptable v/c ratio of 0.89 during the AM peak hour; however, the site impact at this location is less than 1.0 percent (0.010) and no mitigation is required.
- 5. Consistent with background conditions, the West Montgomery (MD 28) and West Gude Drive intersections along Research Boulevard would continue to operate beyond the acceptable v/c thresholds of 0.89 during the AM and/or PM peak hours, and the site impact is projected to be greater than 1.0 percent at both locations. Therefore, mitigation is required at both of these study intersections as a result of the proposed site redevelopment.
- 6. The West Montgomery Avenue (MD 28) intersection at Hurley Avenue could potentially realize increases in turning movement volumes during the PM peak hour as a result of the proposed redevelopment that would result in v/c ratios above the acceptable threshold of 0.89. Since the projected site impact would result in a v/c increase of less than ten percent between background and total future conditions, no mitigation is required per the CTR Guidelines.
- 7. All other study intersections would continue to operate at or within the congestion standards during each peak hour studied under 2019 total future conditions that include the additional site traffic that would be added throughout the area road network as a result of the proposed redevelopment.

- 8. Mitigation measures are proposed by the Applicant at each of the two (2) intersections described above where mitigation is required per the CTR Guidelines.
- 9. At the MD 28 / Research Boulevard intersection, the Applicant is proposing to reconfigure the Research Boulevard approach, currently consisting of one (1) left turn lane, one (1) shared through-left lane, and one (1) right turn lane, to operate with two (2) left turn lanes and one (1) shared left-through-right lane. This improvement would result in three (3) lanes to accommodate the heavy future left turn volume forecasted during the PM peak period. The Applicant will work with SHA to develop and implement these improvements and the corresponding traffic signal modifications.
- 10. At the West Gude Drive / Research Boulevard intersection, the Applicant is proposing to restripe the southbound Research Boulevard approach to convert the currently striped out section of pavement into a second left turn lane. This improvement was previously planned to be completed by the 4 Research Place development; however, the approvals for that development have recently expired. The Applicant will work with the City of Rockville to develop and implement this improvement and the corresponding traffic signal modifications.
- 11. The proposed mitigation described above (detailed in Section 11.2) would provide adequate mitigation of the projected site impact at the study intersections based on the mitigation requirements set forth in the City's CTR Guidelines.
- 12. In addition to the proposed mitigation measures needed to meet the mitigation requirements set forth in the CTR Guidelines, the Applicant is proposing to construct the following additional improvements. While these improvements are not required per the CTR, the Applicant proposes to work with the City and SHA to provide these improvements in order to improve circulation, traffic operations, and both vehicular and pedestrian access in and around the site.,
 - > Provide a new inter-parcel pedestrian connection to the office development to the north.
 - > Provide a new inter-parcel pedestrian connection to the hotel property to the east.
 - ➤ Provide a separate left turn lane on the site driveway approaching Research Boulevard and restrict the left turn movement during the weekday afternoon commuter peak period due to the heavy traffic on Research Boulevard.
 - ➤ Reconstruct and relocate the existing (non ADA compliant) pedestrian ramp along MD 28. The new ramp will meet current ADA standards and will tie into the sidewalk closer to the existing bus stop located adjacent to the Access Drive intersection.
 - The Applicant proposes to reconfigure the entire segment of the Access Drive between MD 28 and the site driveway to provide two (2) outbound lanes leading to the MD 28 intersection. At the MD 28 intersection, the Access Drive approach would be restriped to provide a separate left turn lane with a leading left turn phase and a shared through-right lane. The traffic signal will also be updated to provide a protected left turn movement from MD 28 onto the Access Drive to match the opposing protected left turn phase that was recently implemented at the intersection. As shown in Section 11 of this report, the improvements along the Access Drive and at the intersection would provide significant improvements in side street operations while maintaining adequate mainline levels of service.

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- 13. The proposed additional improvements described above (detailed in Section 11.3) would provide noticeable improvements to both vehicular and pedestrian operations in and around the site.
- 14. A Transportation Improvement Fee is required based on the baseline trip generation estimates detailed in Sections 1 and Section 11 of this report.

Questions regarding this document should be directed to Wells + Associates. o:\projects\6001-6500\6267 research boulevard\documents\reports\revised submission (3.15.2016)\research row ctr (revised submission 3.15.2016).docx



CITY OF ROCKVILLE BOARDS AND COMMISSIONS

NEW MEMBER TRAINING AND ORIENTATION MATERIALS

TRAFFIC AND TRANSPORTATION COMMISSION



City of Rockville Traffic and Transportation Commission

ORIENTATION AND TRAINING MATERIALS FOR NEW MEMBERS

Purpose of the Traffic and Transportation Commission

The Traffic and Transportation Commission advises the Mayor and Council, the Planning Commission, the City Manager, and City staff in matters relating to transportation. As cited in the resolution creating it, the Commission concerns itself with "the establishment of a sound balance of transportation in the City so that the movement of people and goods becomes safe, convenient, and beneficial to all concerned."

Among the Commission's functions are:

- Encouraging diversity in transportation and integration between modes to help reduce dependence on single-occupant vehicle travel.
- Recommending and reviewing transportation projects and programs.
- Assisting the Mayor and Council in mobilizing support for transportation projects of benefit to the City, including at locations not under City jurisdiction.
- Development of policies for the application of traffic control devices supplementary to the Manual on Uniform Traffic Control Devices.
- Development of policies for neighborhood traffic control, including speed control and volume reduction.
- Development of policies to reduce and manage traffic congestion and to enhance pedestrian and bicycle safety.
- Review of the transportation-related aspects of proposed site plans, subdivisions, zoning changes, annexations, etc.
- Review of transportation-related modifications to the Master Plan and Neighborhood Plans, and related oversight of the City's functional classification of streets.
- Review of proposed modifications of the Rockville City Code relating to Streets (Chapter 21), Traffic (Chapter 23), and aspects of the Zoning Ordinance (Chapter 25) related to transportation and parking.
- Oversight of bus and rapid transit and shuttle bus planning within Rockville.
- Development and review of parking policies and standards, both on-street and off-street.
- Development and review of pedestrian policies and programs.
- Development and review of bicycle transportation policies and programs.

• Encouraging coordination between transportation engineering, enforcement, and education.

Authority and Focus

The Traffic and Transportation Commission has no legislative or executive authority and its recommendations are advisory only. The functions under the Commission's purview - as described above - are not exclusive to the Commission but may also be exercised by certain other boards and commissions as well as by City staff.

The Commission's emphasis is typically on policy, broad strategy, and general oversight rather than on details. Outside of regular Commission business, members are welcome to bring location-specific requests, suggestions, and complaints to the attention of the staff liaison.

Members' General Responsibilities

The most fundamental and important responsibility of each Commission member is to regularly attend and participate in the monthly meetings (see attendance policies below). While some members have expertise and experience to lend to the matters which come before the Commission, the Mayor and Council is more interested in receiving input from the "average" citizen. You are not expected to be an expert, and matters requiring engineering expertise or judgment should be referred to appropriate professional staff. What is most important is your willingness to serve and provide input on the many important issues which come before the board.

Meetings

The Traffic and Transportation Commission meets on the fourth Tuesday of each month, at 7:30 p.m. Meetings are usually held at City Hall. Some meetings may be scheduled at other times and/or locations. Most meetings last approximately two hours.

From time to time special meetings or events are held which take place in addition to the regularly scheduled monthly meetings.

Quorum Requirement: A quorum - a majority of the total Commission membership at the time - is necessary in order to conduct official business, take action on motions, etc.

Attendance

Commission members are asked to regularly attend all meetings. Excessive absenteeism, excluding illness or necessary travel, is cause for removal of a member. Three unexcused absences will be considered as a resignation from the Commission. Commission members should notify the staff liaison or chairperson of planned absences prior to the meeting missed.

Length of Term and Reappointment

Commission members are appointed for three-year terms and may apply for reappointment. Members will be notified by the City Clerk's office of their term expiration 60 to 90 days in advance. Those who wish to apply for another term may do so by informing the City Clerk.

Compensation

Commission members serve on a volunteer basis without compensation.

Communications

A briefing packet will be emailed to each Commission member a few days prior to each meeting, as well as posted on the City's website. The briefing packet contains, among other items, the meeting agenda, background materials pertaining to agenda items, and monthly reports on engineering and planning activities. The chairperson receives the agendas and minutes of the Mayor and Council meetings and is expected to keep all members apprised of actions which may be of interest to the Commission.

The Mayor and Council transmit requests to the Commission for recommendation or action through the City Clerk or staff liaison. The Commission transmits its findings, recommendations, or reports to the Mayor and Council via the staff liaison and the City Clerk. Copies of all correspondence to the Mayor and Council should also be sent to the City Manager, the Director of Public Works, and/or the Director of Community Planning and Development Services, as appropriate.

The Commission communicates with the Mayor and Council via monthly meeting minutes, by memorandum, or face-to-face in works sessions or at drop-in sessions. Individual Commission members may contact the Mayor and Council regarding any matter, but is important for members to clarify whether they are expressing personal concerns or opinions, or representing the majority or consensus of the full Commission. Advertising and working with the Planning Commission, the Traffic and Transportation Commission communicates through its own staff liaison and that of the Planning Commission.

The Traffic and Transportation Commission may also advise the City Manager and City staff on matters failing under their respective authority. Recommendations can be made verbally through the staff liaison (to be recorded in the minutes), or through separate memorandum.

Reporting Relationships: Mayor and Council-Commission-Staff

The Traffic and Transportation Commission reports directly to the Mayor and Council; thus its highest business priority should be assigned to Mayor and Council requests and directives.

The staff liaison, via the City Manager, serves as the Mayor and Council's representative to the Commission. The staff liaison is responsible for keeping the City Manager and the Mayor and Council informed through the department head of all Commission activities and decisions.

Commission members and the staff should nurture a shared sense of purpose and work in concert. The staff liaison is assigned to assist the Commission in many ways (see section below), but is not a subordinate of the Commission. While the Commission does not have the authority to issue direct orders to the staff or project work, reports, budgetary decisions, etc., such tasks are often undertaken on a mutually agreed-upon basis. If consensus cannot be achieved on the need for a particular project the Commission desires, this should be communicated to the Mayor and Council, who may at its discretion direct the staff to produce the report.

In actuality, in the interest of getting the important work of the Commission done, and to best serve Mayor and Council's needs, communication goes all ways. The Commission and the staff make every attempt to reach consensus on most issues, and communicate that consensus to the Mayor and Council.

The following outlines the reporting relationships among the Commission, Mayor and Council, and the staff: Mayor and $Council \rightarrow City$ $Clerk \rightarrow City$ $Manager \rightarrow Board \rightarrow Staff$ Liaison.

Work Sessions

The Mayor and Council hold annual work sessions with each board and commission as well as other meetings when circumstances dictate. These work sessions serve as opportunities for open dialogue regarding matters of mutual concern and to address issues which may have, budgetary or policy impacts.

Meeting Minutes

Minutes of all meetings will be taken by the staff liaison or his/her designee. Minutes provide an important record of Commission actions and serve as a communication tool, keeping the City Manager, City Clerk, and the Mayor and Council informed of the proceedings at monthly meetings.

A draft of the previous month's minutes are distributed with the agenda for the next month's meetings. Commission members should review the draft and come prepared to suggest any changes or corrections.

The Mayor and Council has directed that board and commission meeting minutes be prepared in a *succinct* format. Minutes are to primarily record the <u>actions</u> of each commission. When recording discussions in which no motion is made, the general consensus arising out of the discussion is recorded, along with recommendations for future actions, and who is responsible. Commission members who desire more detailed records of discussion points are encouraged to take their own notes.

Chairperson's Role

The chairperson is appointed to a one-year term by the Mayor. Usually the chairperson position is rotated after one or two years so that different members have the opportunity to serve in this role.

The following are examples of the special duties and responsibilities of the chairperson.

- Lead meetings. Ensures that meetings begin and end on time. Most boards and Commissions meetings are run under Robert's Rules, some more formally than others.
- Plan meeting agendas in cooperation with the staff liaison. Encourages participation by all members at meetings.
- Serve as a primary contact to the City Clerk's Office, staff liaison, and the Mayor and Council. Keeps the City Clerk and Mayor and Council abreast of important issues or problems with the Commission.
- Make recommendations to Mayor and Council for appointments and reappointments of members to the Commission.

The Role of the City Manager's Office

The City Manager's Office coordinates all staff activities for boards and commissions. The City Manager is represented at all board and commission meetings by a staff liaison that is appointed by and responsible to the City Manager through the appropriate department head. The staff liaison's principal role is to provide technical support to the Commission and to facilitate the flow of information between the Mayor and Council and the Commission. He or she is also responsible for keeping the City Manager informed, through the department head, of all Commission activities. (See section below on the **Role of the Staff Liaison**.)

THE ROLE OF THE CITY CLERK'S OFFICE

The City Clerk's Office coordinates many member activities, communication to and from the Mayor and Council and citizens, as well as Commission appointments. The Clerk's Office provides information and interpretations on the role of Commission members, Commission operating policies, appointment procedures, and other questions which arise. Official minutes of meetings and other records are also kept by the Clerk's Office.

The City Clerk will keep the Commission and staff liaison abreast of membership activities by providing copies of all relevant correspondences. A directory of elected and appointed officials and members of all boards, commissions, and committees is also maintained by the City Clerk. The most recent copy of this directory is included.

Staff Liaison's Role

The Chief of Traffic and Transportation (Department of Public Works), or his or her staff designee, serves as the chief staff liaison to the Traffic and Transportation Commission, and is responsible for assisting the Commission in a variety of ways. Responsibilities include:

- Assist in developing the monthly meeting agendas.
- Inform members of items of special interest (periodicals, etc.).
- Ensure the Commission's work complements community goals and that the Commission remains focused on its mission.

- Provide information on Mayor and Council and Planning Commission agenda items or decisions that are of interest to the Commission.
- Research and provide background information and analysis on issues under consideration by the Commission.
- Draft letters, memorandums, and other items of communication, as requested by the Commission.
- Provide for technical assistance meeting minutes, copying, requesting information from other City departments, etc.

In consideration of the planning issues under the Commission's purview, the Department of Community Development also provides a representative to each monthly meeting.

Budget

While the Traffic and Transportation Commission advises the Mayor and Council and staff on important budgetary and fiscal matters, the Commission has no budgetary authority per se. A small amount is reserved each year in the Public Works budget to support the special Commission activities such as conferences and tours.

PUBLIC ETHICS ORDINANCE

Chapter 16, "**Public Ethics**," of the Rockville City Code requires that members of boards and commissions disqualify themselves from participating in any decision or recommendation by which they, their immediate family, their business associates, or a business entity in -which they have an interest would be directly and economically impacted. In addition, such individuals are required to file a written statement with the City Clerk disclosing any interest or employment, the holding of which would require disqualification from participation, sufficiently in advance of any anticipated action to allow adequate disclosure to the public.

Board and Commission members must disclose in a statement filed with the City Clerk receipt of gifts during the calendar year in excess of \$25 in value or a series of gifts totaling \$100 or more from entities doing business with the City.

Questions or issues of ethics which arise during a member's term should be referred to the staff liaison or the City Clerk for clarification or guidance.

Lobbying

Recommendations for lobbying efforts are to be referred to the Mayor and Council for review and approval.

Training

At the request of members, or at the suggestion of staff, a variety of training opportunities can be made available. Some commissions have brought in a trainer, others have taken advantage of

videotapes. Types of training sessions which have been requested include 1) How to run an effective meeting; 2) Group dynamics; and 3) Robert's Rules of Order.

Training sessions may also be arranged to cover technical subjects or issues important to the Commission's work. Suggestions for training should be forwarded to the staff liaison or City Clerk.

Open Meetings Law

The State of Maryland has a comprehensive Open Meetings Law. The legislative policy of the statute declare that:

It is essential to the maintenance of a democratic society that, except in special and appropriate circumstances ... public business be performed in an open and public manner; and ... citizens be advised and aware of...the performance of public officials; ... the deliberations and decisions that the making of public policy involves.

The law requires public bodies meet in open session when performing an advisory, legislative, or quasi-legislative function. This law fully applies to Rockville's advisory boards and commissions, including the Traffic and Transportation Commission.

A public body may only close a meeting for one or more of the fourteen enumerated reasons and must limit the discussions to those topics (see appendix). In general, both State law and City policy dictate that Traffic and Transportation Commission meetings should be open to the public in nearly all situations. The Commission is urged to seek the advice of the staff in any situation where closing a meeting is being considered.



Traffic and Transportation Commission Minutes Black Eyed Susan Conference Room Meeting No. 01-16 Tuesday, February 23, 2016 at 7:30 PM

Commissioners Present: Jude Abanulo, Garrett Clemons, Thomas Gibney, Cynthia Griffiths, Gerald Holtz,

Alan Kaplan, Jeremy Martin, Mike Stein, and Melvin Willis

Guest: Janet Wilson, Pat Harris, Mike Workosky, William Zeid, Rick Lundregan, Frank Poli,

Mark Silverwood.

City Staff: Brian Wilson, Katie Mencarini, Daniel Seo

1. General Announcements, Introduction of Guests and Public Comment Period

- a. Commissioner Martin called the meeting to order at 7:30 p.m.
- b. All attendees introduced themselves.

2. Election of New Chairperson

a. Commissioner Gibney made a motion, seconded by Commissioner Griffiths, to elect Commissioner Abanulo as chair. This motion passed unanimously.

3. 15931 N. Fredrick Road (CarMax) Development: Staff Introduction:

a. Ms. Mencarini introduced the proposed development.

4. 15931 N. Fredrick Road (CarMax) Development: Applicant Presentation

a. Mr. Workosky, Wellls & Assoicates, presented the traffic impact study including existing conditions, background developments, trip generation and site accesses, future conditions and mitigations.

5. 15931 N. Fredrick Road (CarMax) Development: Commission Discussion

- a. The Commission discussed transportation elements of the proposed development including transit trip reduction, site accesses, proposed parking spaces, intersection capacity and overcapacity of Shady Grove Metro.
- b. Commissioner Willis made a motion to recommend approval of the proposed development with conditions. The motion was seconded by Commissioner Griffiths. The motion passed 6-3-0 with Commissioners Gibney, Holtz, and Kaplan opposing.
- c. Conditions, outlined in motion to recommend approval are as follows:
 - i. The applicant should accelerate the proposed lane use change (lane re-stripping and signal modification) on WMATA access road at MD 355.
 - ii. The applicant should consider "No Construction Truck" signs at the entrances of the City roadways along MD 355, when the construction begins.

6. Intersection of Monroe Street and Monroe Place: Commission Discussion

- a. The Commission reviewed the police report regarding a recent pedestrian crash at the intersection and discussed possible options to improve safety including a curb extension on Monroe Place.
- b. Ms. Janet Wilson stated that she raised her safety concern at the intersection in the past.
- c. The Commission asked City staff to look into possible improvement options.

7. County's School Bus Depot at 850 Hungerford Drive: Commission Discussion

- a. The Commission discussed residents' concerns regarding the County's plan to relocate their Shady Grove school bus depot to the Carver Educational Services Center at 850 Hungerford Drive.
- b. The Commission discussed possible traffic capacity issues and pedestrian safety issues due to the proposed development.
- c. Commissioner Gibney made a motion, second by Commissioner Griffiths, to add its voice to the concerned citizens who spoke at the February 22 Mayor and Council meeting and to urge the Planning Commission to thoroughly review the application once it is submitted and send the transportation elements to the Commission for review. The motion passed unanimously.

8. Review and Approve October 2016 Meeting Minutes

a. Commissioner Gibney made a motion to approve the October 2016 meeting minutes and Commissioner Holtz seconded the motion, which passed 5-0-4 with Commissioners Clemons, Martin, Stein, and Willis abstaining due to absence at the October meeting.

9. Staff Report and Updates

a. Staff provided the Weekly Report.

The meeting was adjourned at 9:45 p.m.